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A Groundwater Model of Cache Valley, Utah

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A GROUNDWATER MODEL OF CACHE VALLEY, UTAH

by

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ABSTRACT

This report describes the development, calibration and use of a quantitative, predictive management model for the groundwater in the Utah portion of the Cache Valley in northern Utah. The quasi-three-dimensional finite difference computer model was adapted from the U.S. Geological Survey's Trescott and Larson model and simulates the groundwater levels and flows in the groundwater basin. The variable spacing grid system is 23 nodes x 38 x 2 and represents the complex natural system by a simpler approximation with one unconfined and one confined aquifer and the appropriate boundary and initial conditions. River nodes, spring nodes, and constant head nodes were developed to simulate the real interactions of the aquifers with streams, springs, and reservoirs. Evapotranspiration from the land surface is also represented by the program. The program was calibrated by adjusting the model parameters such as transmissivity, storativity, leakance, river nodes, spring nodes, etc. until the predicted values of head and flow were nearly the same as the observed conditions. Calibration was done in two stages: first a steady-state comparison with 1969 conditions and then a transient-state comparison with the water level change maps of 1969 to 1972. The calibrated model was then exercised to predict the groundwater system response to various assumed scenarios of groundwater recharge and draft. Thus the model provides a tool to guide future groundwater development and management towards the best alternatives.

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INTRODUCTION

Groundwater is potentially a major source of water supply in Cache Valley, Utah and Idaho; but presently it is only lightly utilized. Mostly surface water has been used in the past, but as the demand for water is increased by a rapidly expanding population, more industries, and increased irrigation, the groundwater resource will be more fully developed. For planning and management during this development period, knowledge of the groundwater systems must be expanded.

Numerical modeling provides a quantitative tool for groundwater management applications. It helps structure our understanding of groundwater systems and can be used in studying groundwater problems. A model mathematically describes aquifer behavior and is used to simulate aquifer response to various well locations and pumping rates. The methods and tools presented in this study are applicable to similar groundwater problems in other locations of Utah and elsewhere.

This report describes application of a quasi-three-dimensional finite-difference computer model to simulate the groundwater levels and flows in the

Utah portion of the Cache Valley, a major groundwater basin in Utah. Cache Valley was selected for the study because an operational groundwater model is needed during the developmental period to help state officials evaluate proposed withdrawals from the groundwater basin as well as possible recharge methods and other management schemes. From information obtained by modeling, development can be guided towards more complete and economic utilization of groundwater in conjunction with surface water resources.

The computer program used is the U.S. Geological Survey's Trescott and Larson Model (1975, 1976). Some modifications were made to this program in order to adapt the model to the groundwater conditions in Cache Valley.

Due to funding limitations and lack of participation by Idaho agencies, this model study includes only the Utah portion of Cache Valley. The northern boundary of the area modeled is at the Utah-Idaho state line, but boundary conditions were generated to represent the groundwater inflow from Idaho into Utah.

GROUNDWATER CONDITIONS IN CACHE VALLEY IN UTAH

Cache Valley is a north-south oriented structural valley in northeastern Utah and southeastern Idaho (Figure 1). Cache Valley is surrounded by mountains which consist of the Bear River Range on the east, the Bannock, Malad, and Wasatch Ranges on the west, the Portneuf Range on the north, and South Hills on the south. The valley floor, which ranges in altitude from about 4,400 to 5,400 feet above mean sea level, is approximately 60 miles long and mostly 8 to 16 miles wide. Of the approximately 660 square miles in the valley, about 365 are in Utah and 295 are in Idaho. The floor is a low flat plain bordered by gentle alluvial slopes, terraces, and deltas left by ancient Lake Bonneville, and alluvial fans (Bjorklund and McGreevy 1971). The Cache Valley drainage basin, a segment of the Bear River Basin, includes approximately 1,840 square miles, with about 1,180 in Utah and 660 in Idaho, mostly of mountainous terrain.

Geology

Cache Valley is bounded by north-striking, high angle normal faults and is composed of downthrown fault blocks covered by deposits of Cenozoic age. In parts of the valley, the maximum vertical displacement of the fault blocks probably exceeds 10,000 feet. A gravity survey of Cache Valley indicates a maximum thickness of Cenozoic rocks of about 8,000 feet. The bedrock of the Cache Valley watershed consists of Precambrian, Paleozoic, and Tertiary rocks of limestone and dolomite, shale, sandstone and conglomerate, quartzite and phyllite, and volcanic tuff (Bjorklund and McGreevy 1971 and Beer 1967).

Cache Valley was a bay of ancient Lake Bonneville during a part of Pleistocene time. The name "Lake Bonneville" is given to the lake that occupied the basin during the last major glacial state, the Wisconsin, which began about 75,000 years ago (Bjorklund and McGreevy 1971). The valley fill deposits include unconsolidated quaternary, clastic sediments of gravel, sand, silt, and clay, which were derived from the surrounding watershed, and precipitates accumulated in former Lake Bonneville. The pre-Lake Bonneville, Lake Bonneville, and post-Lake Bonneville group of sediments of the Quaternary age form the Cache Valley fill. (See the geologic map and section on Plate 4 of Bjorklund and McGreevy 1971.) The alluvial fan gravels of the pre-Lake Bonneville deposits are exposed along the foothills on both east and west sides of the valley, particularly at the mouth of Blacksmith Fork and Logan Canyons. The Lake Bonneville group is divided into the Alpine, Bonneville, and Provo formations. Each represents deposits of Lake Bonneville at different stages, consisting of lacustrine gravel, sand, silt and clay, that are exposed extensively throughout the valley floor. The Provo formation includes an older gravel and sand member, and a younger silt and clay member. The gravel and sand member forms many deltas, bars, and spits. These deltas form important groundwater recharge zones in Cache Valley. The post-Lake Bonneville deposits consist of slope wash and flood-plain alluvium, and high-level alluvial fan gravels deposited along the mountain fronts, and overlie the silt and clay member of the Provo formation. The sandy flood-plain alluvium is exposed along the major streams of the valley, particularly the

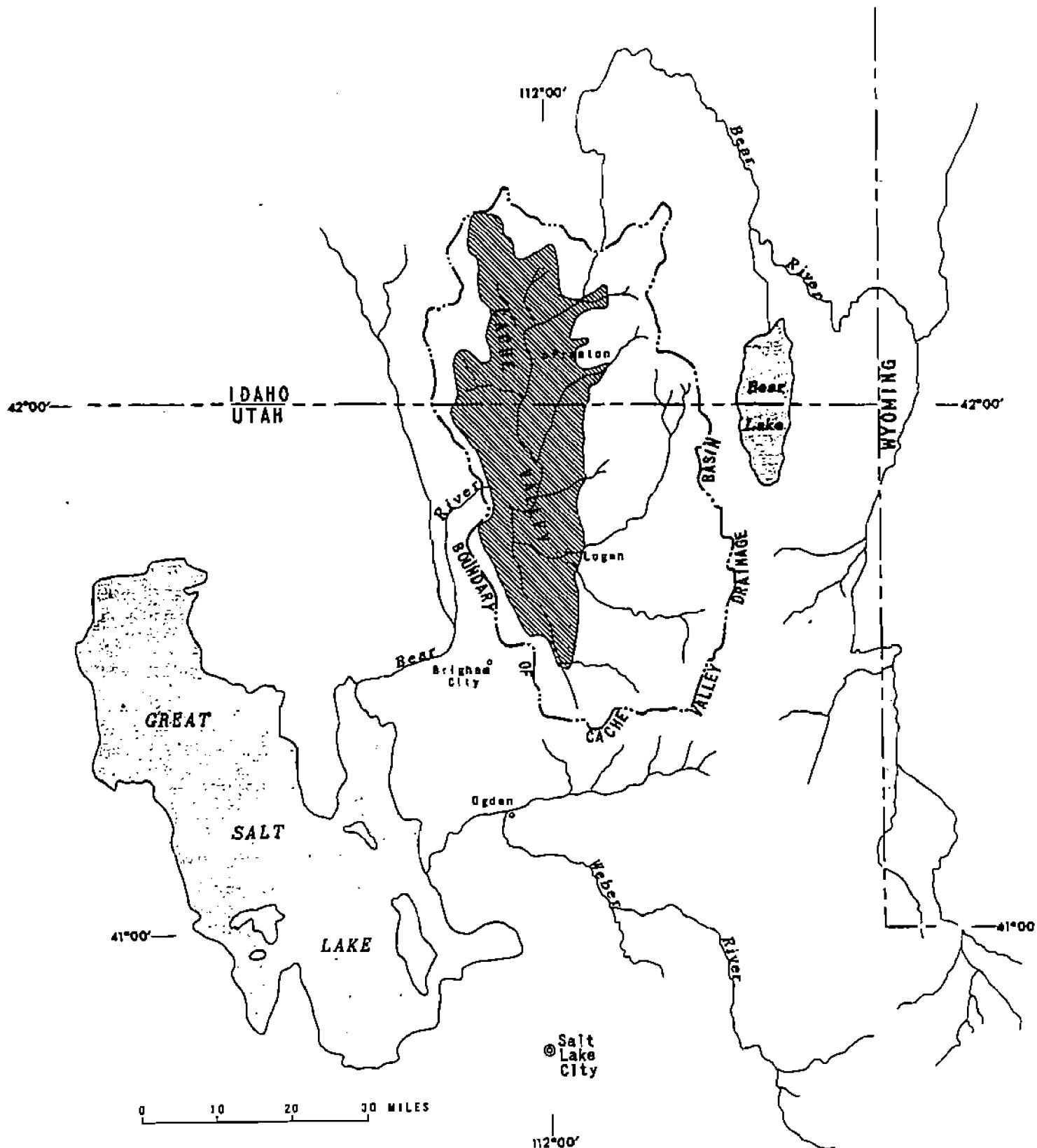


Figure 1. Location of Cache Valley, Utah and Idaho, and the Cache Valley drainage basin (from Bjorklund and McGreevy 1971).

Bear, Cub, Little Bear, and Logan Rivers. The slope wash is exposed on the west side of the valley near Trenton, and along the south flank of the valley near Wellsville and consists of silts and sands (Beer 1967). Two referenced reports by Williams (1958, 1962) give more geologic details about Cache Valley.

In Cache Valley, deposits related to Lake Bonneville and earlier lakes play an important role in the occurrence of groundwater and the many related topographic features affect the occurrence and movement of groundwater.

Hydrology and Climatology

The best sources of groundwater in Cache Valley are from the unconsolidated deposits. Some locations have a water table and others artesian conditions. The general relation of confined, unconfined, and perched groundwater in Cache Valley is illustrated in Figure 2. The diagram directly represents conditions near Logan, Utah, and generally applies to the entire valley (Bjorklund and McGreevy 1971).

Confined groundwater underlies more than 200 square miles in Cache Valley and occurs in those locations where permeable water-bearing aquifers of sand and gravel are overlain by relatively impermeable beds of lake-bottom clay and silt. Within about 130 square miles of the area, the artesian pressure is great enough to force water above the land surface, causing wells to flow. Only a few square miles of this area are in Idaho, most are located in Utah (Bjorklund and McGreevy 1971, plate 3). The confining beds retard upward movement of the water and maintain it under artesian pressure caused by the higher elevations of the recharge areas along the sides of the valley. The confining beds are partially permeable, however, and the groundwater must be modeled as occurring under leaky-aquifer conditions.

Along the mountain fronts near the margins of Cache Valley, the groundwater is unconfined along a narrow strip where the confining beds that overlie the principal aquifers are discontinuous or absent. The boundary of the confining layer is generally gradational rather than abrupt.

In most of the area of flowing wells, a water table in an aquifer above the confining layer is near the land surface. The shape and the slope of the water table is generally about the same as that of the land surface. Local perched groundwater bodies are also common in many parts of Cache Valley. They develop above the main water table where beds of clay or other materials of low permeability intercept water percolating downward (see Figure 2). Some of the perched groundwater bodies are seasonal and poorly defined. Their water is generally not tapped by wells, but perched water is often encountered when drilling wells on the alluvial slopes and fans and in excavations for construction (Bjorklund and McGreevy 1971).

The well-defined seasons (warm and wet springs, warm and dry summers, cool and wet autumns, cold and damp winters), large daily temperature changes, and moderate precipitation are characteristics of the climate of the Cache Valley. The growing season ordinarily lasts about 150 days from May through September. Snow usually covers the valley floor during December, January, and February. The valley normally receives 10 to 20 inches of precipitation annually, and 20 to 50 inches fall in the mountainous area surrounding the valley. The runoff usually has its maximum volume during May or June and results mostly from melting snow which accumulated during the winter on the surrounding mountains (Bjorklund and McGreevy 1971 and Beer 1967).

Groundwater Budget Analysis

Over any finite period of time, the quantity of water entering Cache

EXPLANATION

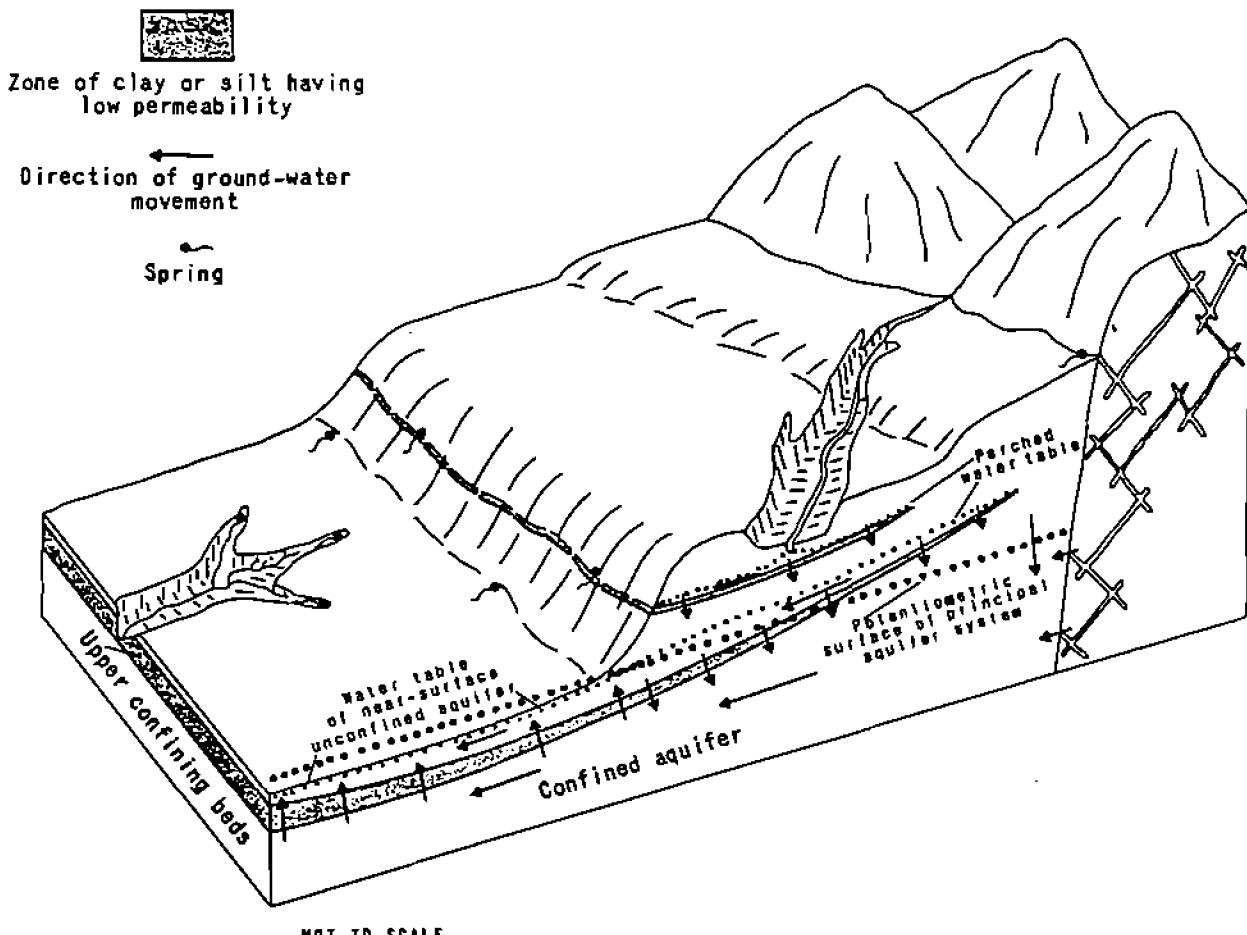


Figure 2. Relation of confined, unconfined, and perched groundwater in Cache Valley (from Bjorklund and McGreevy 1971).

Valley is equal to the quantity leaving the valley plus or minus the change in storage within the valley. Since much of the groundwater reservoir in Cache Valley is overflowing, and the change in groundwater storage over the years has been small or negligible, the total recharge in Cache Valley is about equal to the total discharge and is estimated to be about 280,000 acre-feet per year (Bjorklund and McGreevy 1971). The Utah portion, estimated from the model calibration of this project, is about 170,000 acre-feet annually. Items of inflow and outflow of groundwater are given in Table 1. Deep subsurface outflow from Cache Valley is negligible.

Groundwater Conditions by Areas

Bjorklund and McGreevy (1971) used land use data (see their plate 4) along with geologic and hydrologic conditions to subdivide Cache Valley into 11 principal areas of generally similar groundwater conditions as shown in their Figure 13. Seven of those areas are within the model region as shown in Figure 3. The boundaries are not sharply distinct, and the areas blend into each other. The dotted lines which delimit the areas are not lines of separation but approximations of where general conditions change. Outside of these areas, groundwater occurs locally

Table 1. Groundwater budget analysis, Cache Valley, Utah.

	Utah & Idaho (USGS) (acre-feet)	Utah (acre-feet)
<u>Recharge</u>		
Seepage from Irrigation System	100,000*	72,000
Subsurface Inflow	32,000*	18,000
Seepage from Streams and Infiltration of Rainfall	148,000	80,000
Total	280,000*	170,000
<u>Discharge</u>		
Pumpage (from Wells)	29,000*	24,000
Evapotranspiration	108,000*	63,000*
Accretion of Groundwater to Streams Rivers and Springs	143,000	83,000
Total	280,000*	170,000

*From Bjorklund and McGreevy (1971); other amounts are estimated from the groundwater modeling in this study.

in slope wash and alluvium, in sandstone and conglomerate of the Salt Lake formation, and in fracture and solution openings in older rocks.

The above information on areas of similar groundwater conditions was helpful at an early stage of the study in setting up the simulation model and estimating some parameters, especially for the values of transmissivity and hydraulic conductivity of different aquifer systems. Brief descriptions of the areas of similar groundwater conditions in Cache Valley in Utah are excerpted from Bjorklund and McGreevy (1971) and given below.

Smithfield-Hyrum-Wellsville area

Coarse fan and delta deposits of Summit Creek, Logan River, Blacksmith Fork, and Little Bear River coalesce to form this complex aquifer system in the Smithfield-Hyrum-Wellsville area in Utah

(area 1, Figure 3). This aquifer system is the largest and most productive in Cache Valley and is an overflowing groundwater basin where the shallow water table is so high that groundwater flows from many seeps and springs into the surface water streams. The system is very permeable, and the transmissivity ranges from 10,000 to 330,000 ft² per day. Both confined and unconfined aquifers are present.

Little Bear River area south of Hyrum

This unconfined aquifer system extends southward from Hyrum along the flood-plain and terraces of the Little Bear River (area 2, Figure 3). The groundwater recharge is mostly by seepage from streams and canals and from irrigation. The thin aquifer is very permeable and the estimated transmissivities are up to about 15,000 ft² per day.

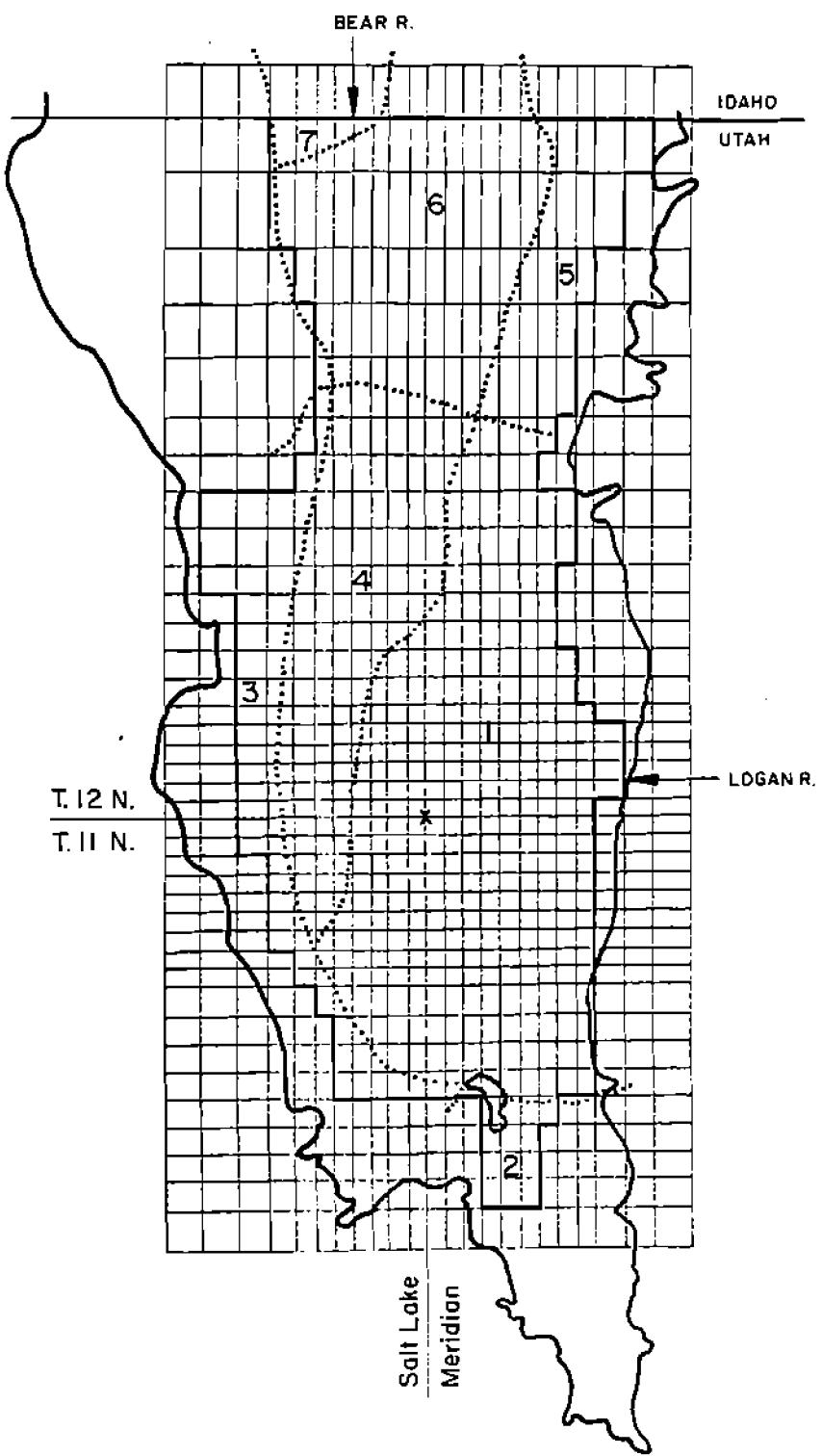


Figure 3. Map showing groundwater areas in the Cache Valley, Utah model.

Wellsville to Newton area

Groundwater conditions are poorly known in this area which lies along the west side of Cache Valley from southeast of Wellsville to near Newton (area 3, Figure 3). Groundwater is both confined and unconfined. The fill is predominantly low permeability and fine grained, but it includes some permeable sands and gravels.

Lower Little Bear River-Benson-the Barrens area

This area is in the central part of Cache Valley along the lower part of the Little Bear river and near Benson and the Barrens (area 4, Figure 3). The Quaternary deposits are predominantly clay and silt but have thin beds of sand and fine gravel that contain confined groundwater. In most of this area, artesian pressures are high, and heads as much as 62 feet above the land surface have been measured. The hydraulic connection between the water-producing beds is poor.

Cub River subvalley area

This aquifer system extends along the Cub River from near Franklin to a few miles south of Richmond (area 5, Figure 3). The groundwater is mostly confined and some wells flow. Transmissivities probably range from

about 1,000 to 4,000 ft² per day in most of the area. The gravel deposits of the Cherry Creek and High Creek alluvial fans are thicker, and their transmissivities are higher.

Fairview-Lewiston-Trenton area

The reworked sand and silt deposited from near Fairview to Cutler Reservoir, as the Bear River eroded into its delta near Preston, formed this principal water-bearing material in the area from near Fairview and Lewiston to near Trenton (area 6, Figure 3). Groundwater is unconfined and is near the land surface. The transmissivity is probably less than 1,000 ft² per day. Potentially productive aquifers may lie beneath the delta deposits in the older deposits, but they have not been explored east of the present Bear River channel.

Weston Creek subvalley area

This is one of the major aquifer systems in the Idaho part of Cache Valley (area 7, Figure 3), and a small portion extends into Utah. The groundwater is both confined and unconfined. The transmissivity is estimated to be about 30,000 ft² per day in the area southeast of Weston. The proportion of permeable water-bearing materials decreases southward and eastward from near Weston.

MODEL DESCRIPTION

A groundwater model represents the heads, flows, and hydraulic losses in a geologic environment with equations containing hydrologic and hydraulic parameters. Computer solution is used to simulate the response of the groundwater system to natural conditions and human development.

Flow in a porous saturated medium in three dimensions may be expressed by the partial differential equation

$$\frac{\partial}{\partial x} (K_{xx} \frac{\partial h}{\partial x}) + \frac{\partial}{\partial y} (K_{yy} \frac{\partial h}{\partial y}) + \frac{\partial}{\partial z} (K_{zz} \frac{\partial h}{\partial z}) = S_s \frac{\partial h}{\partial t} + w(x, y, z, t) \quad (1)$$

where h is the hydraulic head [L] at time, t , K_{xx} , K_{yy} , K_{zz} are the principal components of the hydraulic conductivity tensor aligned with the coordinate axes [L/t], S_s is the specific storage [$1/L$], x , y , z are the coordinate directions [L], and $w(x, y, z, t)$ is a source term for inflow or withdrawal per unit volume of an aquifer. In most cases, analytical solution is not possible for Equation 1. However, a variety of numerical techniques have been developed for use with high-speed digital computers to approximate solutions of partial differential equations. These methods have greatly enhanced capabilities for modeling groundwater flow. The computer program applied in this study uses finite-difference approximations to solve Equation 1.

Computer Program

The finite-difference and finite-element methods are the two major

numerical techniques used to obtain approximate solutions. Most of the available groundwater model computer codes are based on one of these two methods.

Because numerous groundwater computer programs are available today, the first question in beginning a study is, "Which one should be used?" Considerable effort was spent to find a general program which could be easily adapted to portray the Cache Valley groundwater systems. Because of the complex geologic situation in Cache Valley, a three-dimensional model is preferred. Two tested three-dimensional models were available when the study began; one from USGS using the finite-difference method (Trescott 1975 and Trescott and Larson 1976), and the other from the University of California, Davis, using the finite-element method (Gupta et al. 1975).

The finite-element method reproduces the complex geometry of groundwater aquifers more conveniently and more accurately. Often it requires fewer nodal points to represent the discretized aquifer to the same level of accuracy, thus cutting down on storage, execution and input/output costs (Townley and Wilson 1980). However, when this project began, the finite-element computer codes were in somewhat early stages of development. Further work on accuracy, numerical stability and convergence properties, and its consistency with the physical system was needed. On the other hand, the finite-difference program from the USGS was well-documented and easily available to the user. Most important, this code has been widely used and continuously improved to accommodate

various computer systems. After comparing the two, it was decided to use the codes from USGS which are based on the finite-difference method.

The USGS computer program as documented by Trescott (1975) and Trescott and Larson (1976) permits the use of variable grid spacing and uses the strongly implicit procedure (SIP) to solve simultaneously the set of equations which results from the finite difference equation at each grid point in the context of the appropriate boundary and initial conditions over the region of interest. The porous medium in which the flow is to be simulated may be heterogeneous and anisotropic and have irregular boundaries. One or more layers of nodes can be used to simulate each hydrogeologic unit. The uppermost hydrologic unit may have a free surface. Stress on the system may be in the form of well discharge (or recharge) and recharge from precipitation. Changes were made in the program to adapt it for use in the Cache Valley model. For example, river nodes and spring nodes were developed to represent the actual rivers and springs and evapotranspiration simulation was programmed for use in appropriate areas of the model.

Later on the 1983 version of the three-dimensional finite-difference groundwater simulation program was obtained from the USGS. In this improved version the programming has been modified to give increased capability and to be easier to understand. Because of the very large amount of input data needed for groundwater simulation, the USGS streamlined the method to handle the input data. Input to the program is specified in independent files, one basic package with 12 major options. For example: recharge, rivers, drains, and evapotranspiration are major options handled by separate packages (McDonald 1982). Each option has its own input file. A given file is needed only if the corresponding major option is going to be used and is specified by the user.

The Drain package and River package of the 1983 USGS model perform the same function as the spring nodes and river nodes adapted by the project for use in the Cache Valley model. The evapotranspiration package of the USGS model defines a linear relationship between the evapotranspiration rate and the depth below the land surface just as is used in the Cache Valley model (see Figure 12). Two additional solution packages for Equation 1 are included besides the SIP package of the earlier versions. The SSOR package uses the sliced-successive overrelaxation method. The DE4 package, uses a direct solution method, but can only be used for two-dimensional models (one aquifer layer). Thus, the new version of the program has improvements that make it easier and faster to use and make its basic capabilities much the same as used in the Cache Valley model.

The large number of arithmetic calculations and the excessive computer storage required for numerical solution of the three-dimensional groundwater flow equations of multiple aquifer systems usually precludes solving the equations in three dimensions. To adequately represent transient flow in multiple layers, each layer must be represented by a number of nodes. This greatly increases the number of nodes used to represent the system as well as the computer time required for solution. When the hydrologic system can be represented by aquifers in which flow is assumed to be only horizontal between confining layers in which flow is assumed to be only vertical, the fully three-dimensional problem is reduced to a quasi three-dimensional problem by eliminating the layers of nodes representing the confining beds. One can then solve the two-dimensional equations for each aquifer with the aquifers being coupled through their vertical leakage (Bredehoeft and Pinder 1970). In the Cache Valley model, each hydrologic unit is represented by one layer of nodes. For this approach, Equation 1 is

multiplied by b , the thickness of the unit, becomes

$$\begin{aligned} \frac{\partial}{\partial x}(T_{xx} \frac{\partial h}{\partial x}) + \frac{\partial}{\partial y}(T_{yy} \frac{\partial h}{\partial y}) + b \frac{\partial}{\partial z}(K_{zz} \frac{\partial h}{\partial z}) \\ = s \frac{\partial h}{\partial t} + bw(x, y, z, t) \quad (2) \end{aligned}$$

where

T_{xx}, T_{yy} are transmissivity tensor
[L²/t]

s is the storage coefficient
(dimensionless)

For a quasi three-dimensional model, the third term in Equation 2 is replaced by the flow through the confining layer into or out of the adjacent aquifer and is given by

$$q = - \frac{K}{L} (h_{(i,j,k+1)} - h_{(i,j,k)}) \quad (3)$$

where

K is the hydraulic conductivity of the confining layer [L/t]

L is the thickness of the confining layer [L]

h is the hydraulic head in aquifer [L]

k is the index in the z direction

This quasi three-dimensional model minimizes the computer-memory required for the simulation.

Selection of Model Grid System

The construction of the grid system for the model required considerable effort because of the complex geologic conditions in Cache Valley. As a first step in gaining understanding of the groundwater and subsurface geology, a

"peg" model of the basin was constructed on the assembled 1:25,000 topographic maps utilizing the best well logs from the files of the Logan District Office of the Utah Water Rights Division and from the selected hydrologic data reported by McGreevy and Bjorklund (1970) for Cache Valley. Each peg represented one well with information on well depth, well yield, water level, and stratigraphic information. Additional information on geohydrologic sections in Cache Valley, Utah and Idaho, came from a USGS open file report (McGreevy and Bjorklund 1971). Finally some valuable assistance came through consultations with Dr. J. Stewart Williams, a retired geologist, of Logan, Utah.

Data are insufficient for complete definition of the aquifer system with all the individual hydrogeologic units. Local artesian aquifers do not persist laterally, and for all practical purposes cannot be separated. The most efficient and reasonable modeling approach is to combine several hydrogeologic units into a larger unit with equivalent overall storage properties. Thus, for the model, the groundwater system was divided into an artesian aquifer overlain by a confining bed, which in turn was overlain by an unconfined water-table aquifer.

The grid used to model the aquifer is shown in Figure 4. A block-centered, finite-difference grid with variable grid spacing was used. The grid consisted of 23 rows and 39 columns, or 897 nodes for one aquifer layer. The grid spacing ranged from 0.5 mile to 2 miles on a side. Since the quasi three-dimensional approach eliminates the layer of nodes representing the confining bed, two layers of nodes were used, one represents the confined and the other the unconfined aquifer.

In designing the grid system, smaller (or shorter) grid spacings were used in areas where a large number of wells are located, the hydraulic gradient is steep, or the transmissivity

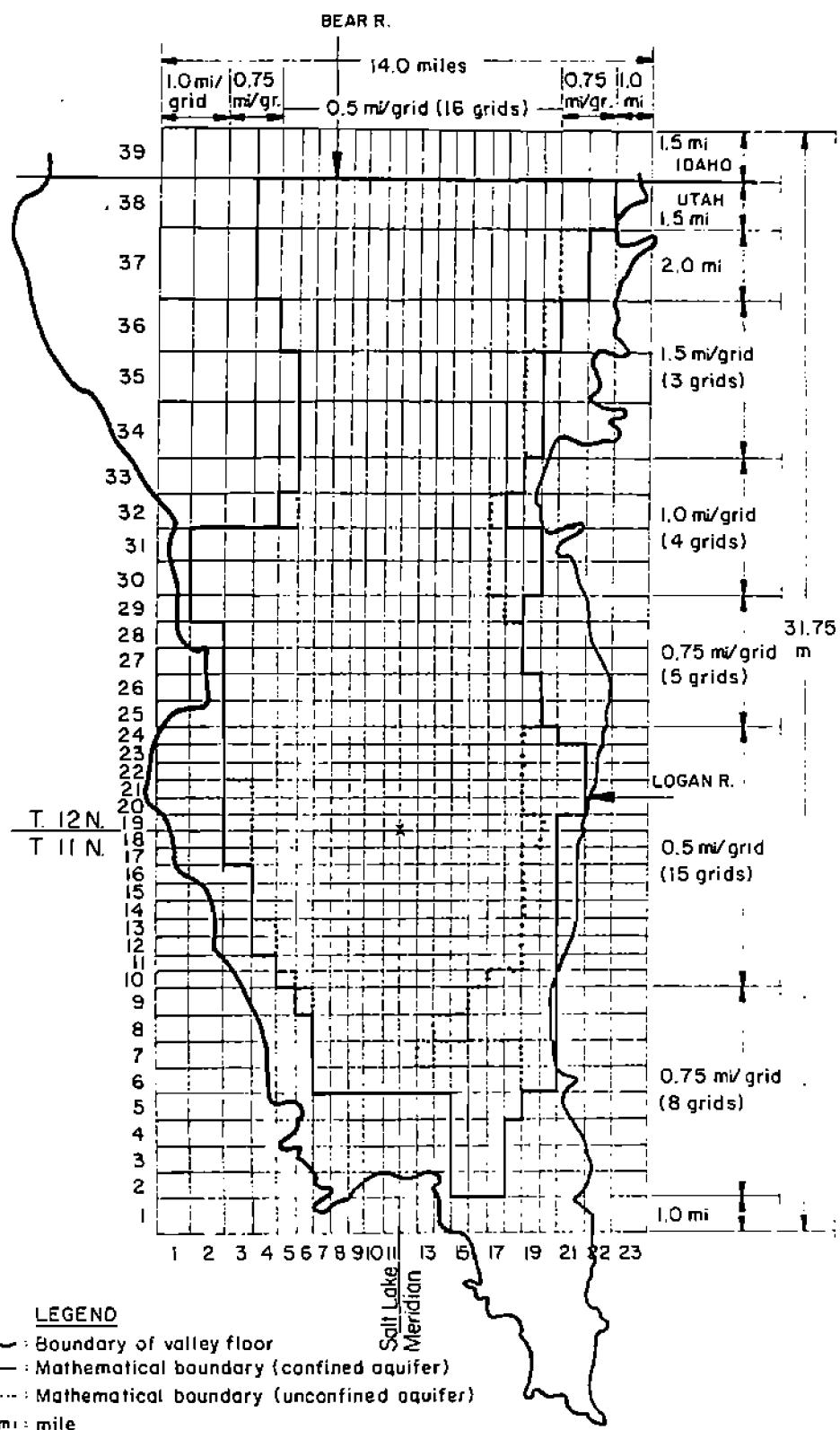


Figure 4. Model grid system and boundaries.

or hydraulic conductivity varies greatly over short distances. A restriction on the spacing ratio $x_j/x_{j-1} \leq 1.5$ between any two adjacent grids was followed in order to reduce truncation errors and resulting convergence problems. The grid system was not changed during the calibration.

Parameters

Required input data were obtained by collection and interpretation from historical data, field measurements, reasonable assumptions and estimations, and by varying values during calibration until a best-fit condition was obtained. Some parameters, such as the type of model, grid system, initial water levels, ground surface elevations, bottom elevations of water table unit, and boundary conditions, were set before the model was constructed and were not varied. Other parameters were changed during calibration. These were transmissivity, hydraulic conductivity (unconfined aquifer), storage coefficient, recharge, leakance (the resistance to vertical flow in the confining bed), maximum evapotranspiration rate, river node coefficients and spring node coefficients.

The values for some parameter matrices were obtained by overlaying the grid on a map of plotted parameter values. For example, the initial head matrix was obtained by overlaying the grid on a map of the initial potentiometric surface and determining the head value at each node. The ground surface elevations, river node elevations, and spring node elevations were obtained from 1:25,000 topographic maps by using the same procedure. Detailed description of how these parameters were obtained are presented in later sections.

Boundary conditions and recharges

The model can represent two kinds of boundaries: constant head and constant flux. Zero-flux boundaries,

where nodes have zero transmissivity (hydraulic conductivity), are set along the entire border of each layer of the model as a computational expediency; and consequently, the flux across the outer border is zero. Non-zero constant head or constant flux boundaries are then placed just inside this border at selected points to represent inflows and outflows. Cutler Reservoir and Hyrum Reservoir are represented by constant head nodes using the local normal depths of the reservoirs as heads. The river channels of the Bear River and the Cub River upstream from their junction to the Idaho-Utah state line are also represented by constant head nodes. In this area these channels are cut deeply into the ancient lakebed sediments and the constant head nodes represent the natural conditions better than would river nodes, and also separate the two adjacent but independent shallow, unconfined aquifers. This representation is adequate so long as the water levels in the rivers do not change greatly during the year.

In Cache Valley, the principal recharge areas are along the margins of the valley and are underlain by permeable unconsolidated materials including beds of gravel and sand. Runoff infiltrates where the streams flow from the canyons onto coarse-grained deposits to recharge the underlying groundwater reservoir. Most of the recharge is from perennial streams that provide a constant source, but some water is contributed by intermittent and ephemeral streams as well as by direct rainfall and snowmelt along the mountain front. Recharge from irrigation water comes from the seepage under irrigation canals and ditches and from irrigated land. Most of the recharge takes place along the edges of the valley where water infiltrates most readily and where much irrigation water is applied. Recharge occurs also by water moving directly into the aquifers from rocks in the adjacent mountains as subsurface inflow (Bjorklund and McGreevy 1971). Recharge from the mountains and other

sources along the edge of the valley were treated as constant finite-flux boundaries and recharge "wells" were placed just inside the no-flow boundary at nodes where the recharge is assumed to take place (see Figure 5). In the lower parts of the valley, some infiltration reaches the shallow unconfined aquifers, but infiltrated water does not reach the confined aquifers because of the upward artesian gradient. The northern boundary of the area covered by the model is the Utah-Idaho state line. According to Bjorklund and McGreevy 1971 about 4,000 acre-feet of groundwater moves annually from Idaho into Utah. This includes 3,000 acre-feet in the area west of Bear River near Weston, Idaho and 1,000 acre-feet in the Cub River subvalley mostly east of Cub River. Recharge wells were assigned at nodes in those areas to simulate the flows.

The amount of water initially assigned to a recharge well was calculated by the equation:

$$\text{Recharge} = T \times L \times H \times 365$$

x conversion factor dependent
on recharge units (4)

where

T is transmissivity [ft² per day]

L is width of the grid cell [ft]

H is the hydraulic gradient [ft per ft]

and reasonable estimates were made for the parameters from hydrologic and hydrogeologic data. These recharge estimates were then adjusted by trial-and-error during calibration.

The bottom of the confined aquifer is assumed to be impermeable to simulate the negligible subsurface outflow from Cache Valley.

Initial water levels for the confined aquifer

In the model, many hydrogeologic units are combined into a single layer confined aquifer with equivalent storage properties. The potentiometric head of the hypothetical combined confined aquifer represents an average for the actual aquifer system.

The potentiometric surface contour map of March 1969 from plate 4 in Bjorklund and McGreevy (1971) is the most complete water level contour map of the entire model area available for calibration. An apparent anomaly, in the form of a mound on the potentiometric surface in the middle of Cache Valley about 5 miles northwest of Logan, probably results from differences in artesian pressure in wells of different depths. Wells in the vicinity of the mound tap sand and gravel artesian aquifers at depths ranging from 480 to 760 feet below land surface, whereas wells in the surrounding area tap aquifers at much shallower depths (Bjorklund and McGreevy 1971). Another shortcoming of this map is that it only shows the unconfined or perched water levels in two areas--the Little Bear River area south of Hyrum and the area between the Bear River and the Cub River channel south of the Idaho-Utah state line.

Because of the deficiencies of the map, adjustments were made based on interpretation of other data to produce the 1969 map shown in Figure 6. The geohydrologic sections related to the model area were examined from the open file report by McGreevy and Bjorklund (1971). Water levels of two wells in the Bear River delta between the Bear River and the Cub River were measured with the help of Mike Turnipseed of the Division of Water Rights, Logan office. All the well information from the Division of Water Rights and from available reports and measurements were carefully studied. The water levels used to draw the contour map of Figure 6

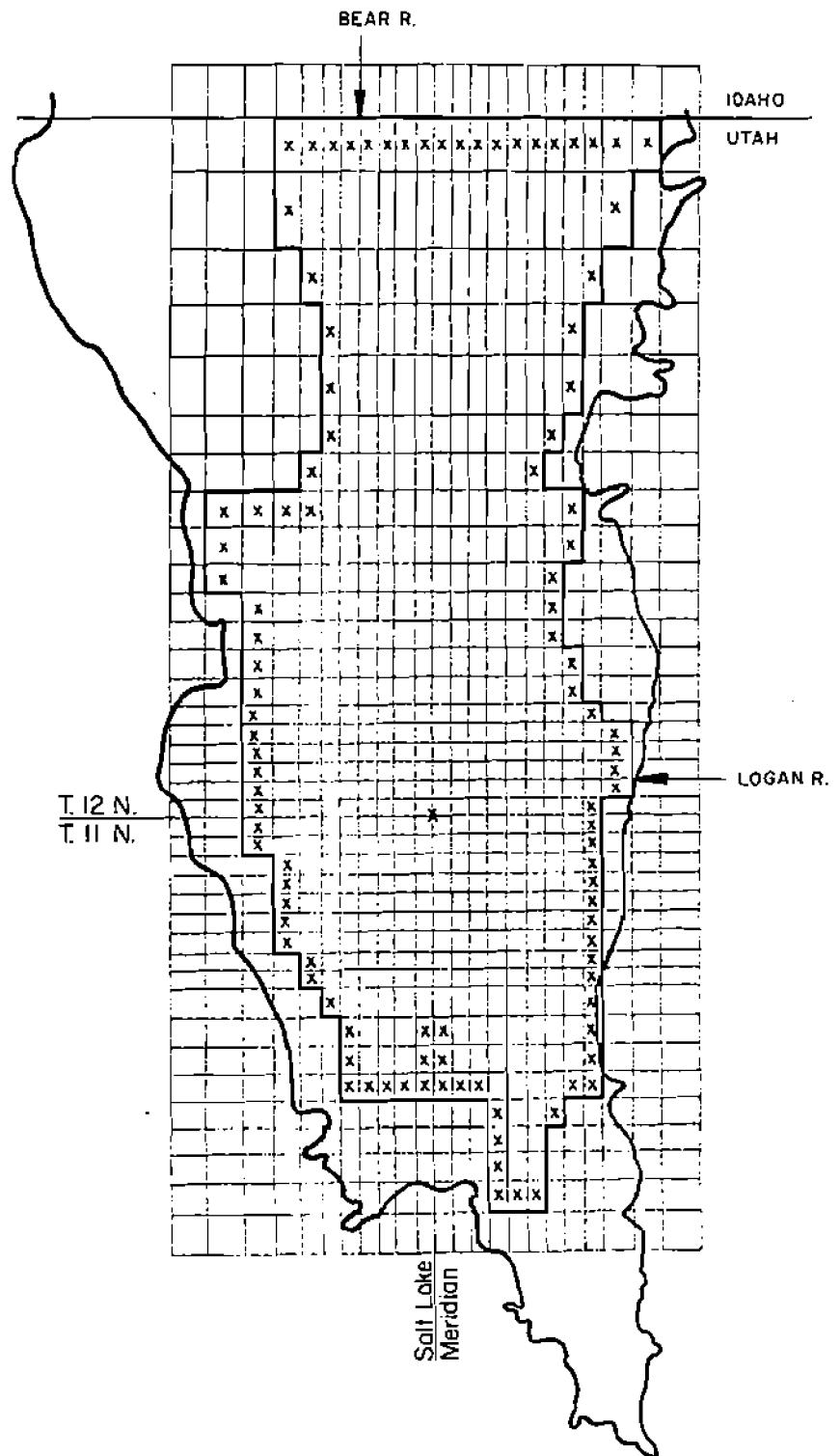


Figure 5. Nodes of recharge wells (constant finite-flux boundaries) for layer 1 confined aquifer.

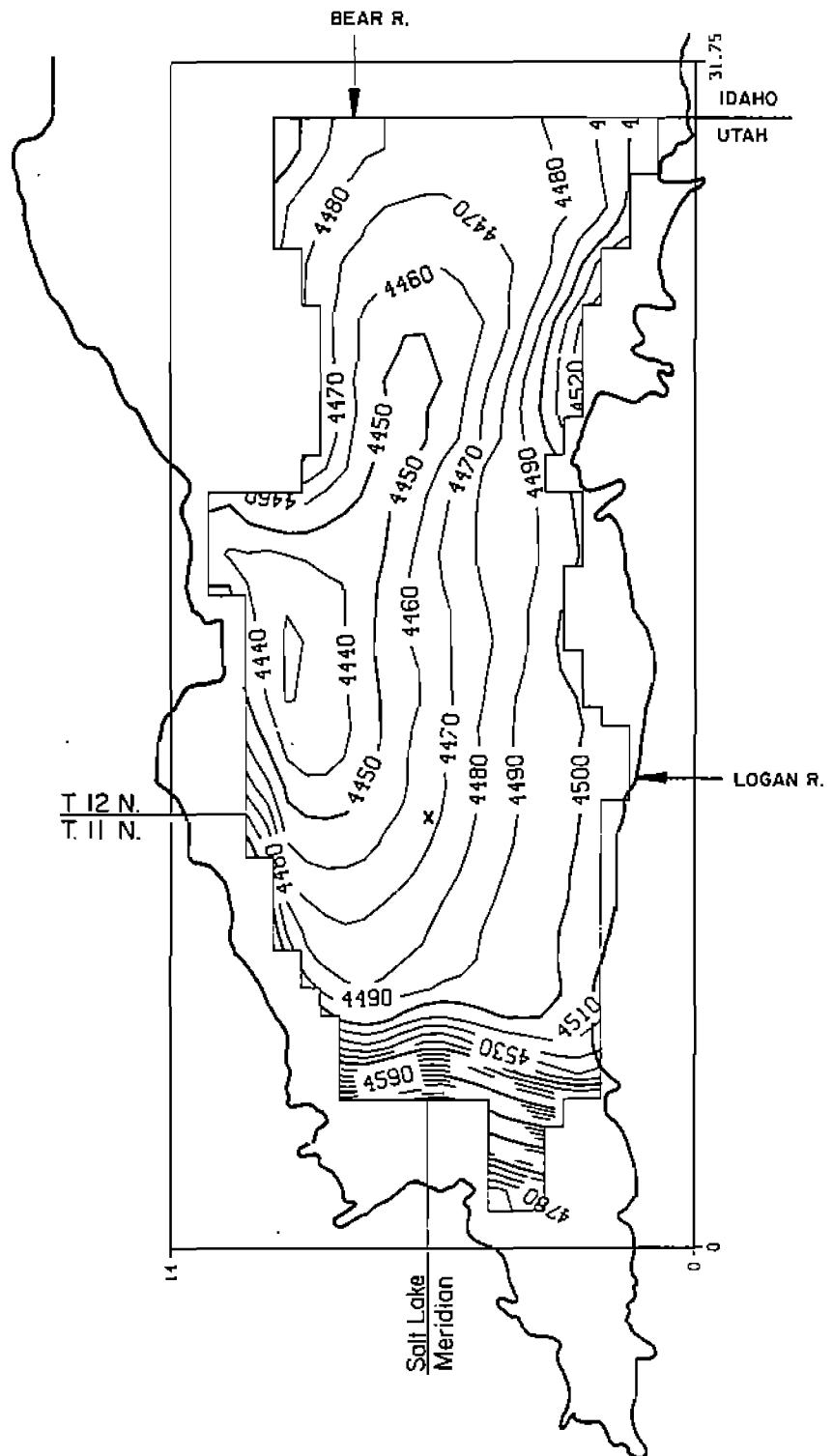


Figure 6. The observed potentiometric surface contour map of the confined aquifer of March 1969 (based on Bjorklund and McGreevy 1971).

have been taken from the wells which demonstrate consistency in depth and flow rate. Thus, the estimates of the potentiometric surface plotted on Figure 6 were based on all the information available as adjusted by consideration of related data and reasonable judgment.

A model grid system map was overlaid on the modified piezometric contour map of Figure 6 and the piezometric head at each node was recorded to give the initial well water levels for the confined aquifer in the model.

Initial water levels for the unconfined aquifer

The geologic conditions in Cache Valley in Utah provided a basis for separating the unconfined groundwater aquifer into three regions. The first region is the area east of Bear River, west of Cub River, and south of the state line. The deposits in this area are the reworked sand and silt as the Bear River eroded into its delta near Preston. In most of the area, the sand and silt is approximately 30 feet thick and covers lake-bottom clays that have very low permeability. Groundwater is unconfined and is near the land surface. The water level contour map in this area is given on plate 4 in Bjorklund and McGreevy (1971). The starting head for the model at each node was estimated by overlying the model grid on the contour map.

The water table is near the land surface in a second region that covers most of the area with flowing wells in Cache Valley (see plate 3 in Bjorklund and McGreevy 1971). This shallow aquifer is partly recharged by water seeping upward from the artesian aquifers and partly by water seeping downward from irrigation canals, ditches, irrigated fields, and infiltrated rainfall. The aquifer is discharged by evapotranspiration, irrigation, springs, and natural and artificial drains. The shape and slope of the water table in this second region is presumed to be

generally about the same as the wet or damp land surface in the artesian areas (Bjorklund and McGreevy 1971). The starting water table head for each node was estimated from the ground surface elevation.

The third unconfined region is the local perched groundwater zone south of Hyrum. This shallow water table is perched in thin deposits of gravel on layers of silt or interbedded clay. The perched water table contour map shown on plate 4 in Bjorklund and McGreevy (1971) was adjusted to obtain initial head values.

A water level contour map including the three different unconfined regions in March 1969 is shown on Figure 7. Nodes between the three unconfined aquifer regions are assigned as constant head nodes in the model and are shown as the dashed lines in Figure 7.

Transmissivity of the confined aquifer

The boundaries of the principal groundwater aquifer and of areas where groundwater conditions are generally similar (Figure 3) were outlined on a grid system base map. The major rivers and streams were also drawn on the same base map. Transmissivity values determined by pumping and recovery tests, as listed in reports or publications or in test records obtained from the Water Rights Division, were collected and marked on the base map according to their locations. Starting from those nodes where the transmissivity values were already known from aquifer tests, estimates were made of the transmissivity values for the nearby areas. In general, transmissivity values decreased from the mountains toward the valley floor, but higher values occurred along rivers and streams.

The transmissivity values range between 330,000 and 1,000 ft² per day over the model area. The initial transmissivity estimates were varied during the calibration of the model

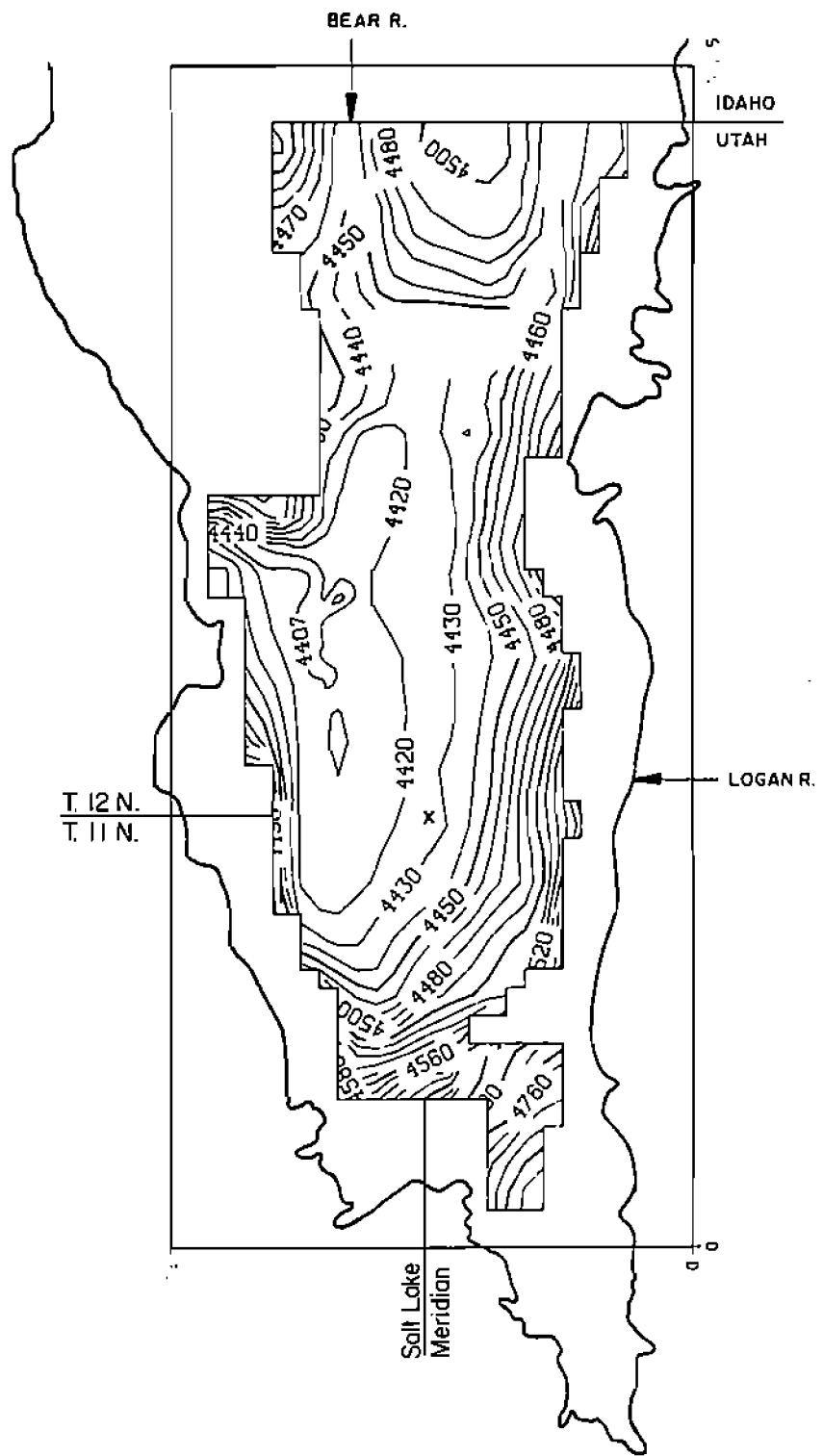


Figure 7. The observed water level contour map of the water-table aquifer for March 1969.

until the set of transmissivities was obtained which minimized the difference between the simulated and observed potentiometric surfaces. Figure 8 shows the transmissivity contours of the confined aquifer as adjusted by calibration of the model.

Hydraulic conductivity of the unconfined aquifer

The geologic map of Cache Valley of Utah and Idaho shown on plate 1 in Bjorklund and McGreevy (1971) delimit the different geologic units for the model region. The description of the composition and water-bearing properties of each geologic unit formation are given in Table 4 in the same document. A grid system base map was overlaid on the geologic map, and the initial values of hydraulic conductivity were then assigned to each node according to the aquifer properties. The calibration procedure was similar to that already described for the confined aquifer and values were in the range from 0.00005 to 0.002 feet per second after adjustment by the calibration. The hydraulic conductivity contour map of the unconfined aquifers with interval value 0.0002 is shown in Figure 9.

Storage coefficient

For steady-state aquifer conditions, no changes occur with passing of time. To represent such conditions during model calibration a storage coefficient of zero was set at all the nodes of all the layers to eliminate the time dependent term in Equation 1. In transient-state calibration, however, initial storage coefficients were assumed and then varied in order to improve the agreement of measured with calculated water levels.

Generally for confined aquifers, the storage coefficient values range between 0.001 and 0.00001, while in unconfined aquifers, the range is 0.5

to 0.05. In the Cache Valley the observed storage coefficients for confined aquifers cover only part of the expected range. The confined aquifer storage coefficients were obtained by a procedure similar to estimate transmissivity values. The estimation process began by marking storage coefficient values from aquifer tests on a grid system base map. Storage coefficient values at nearby nodes were based on geologic and hydrologic information. The values of the storage coefficients so obtained for the confined aquifer range from 0.0001 to 0.0004, as shown in Figure 10.

Since little information was available on storage coefficient values for the unconfined aquifers, an estimated initial value of 0.1 was used for each node in the storage coefficient matrix. During the calibration no changes were made in the unconfined aquifer storativity because the water levels are not sensitive to variation in the unconfined storage coefficients.

Leakage

Since horizontal flow in the confining bed was ignored, the aquifer system was simulated as a quasi three-dimensional model with resultant savings in computer time and storage. The confining beds were not represented by layers of nodes; instead, the flow through the confining bed was incorporated in the vertical components of hydraulic conductivity of the adjacent aquifers. In usual modeling practice where adequate leakage data are available, this is accomplished by computing the trial leakage (TK) values using Equation 5 and then inputting these into the model:

$$TK_{(i,j,k)} = \frac{2(bK_{zz})_{k+1} (bK_{zz})_k}{(bK_{zz})_k Z_{k+1} + (bK_{zz})_{k+1} Z_k} \quad (5)$$

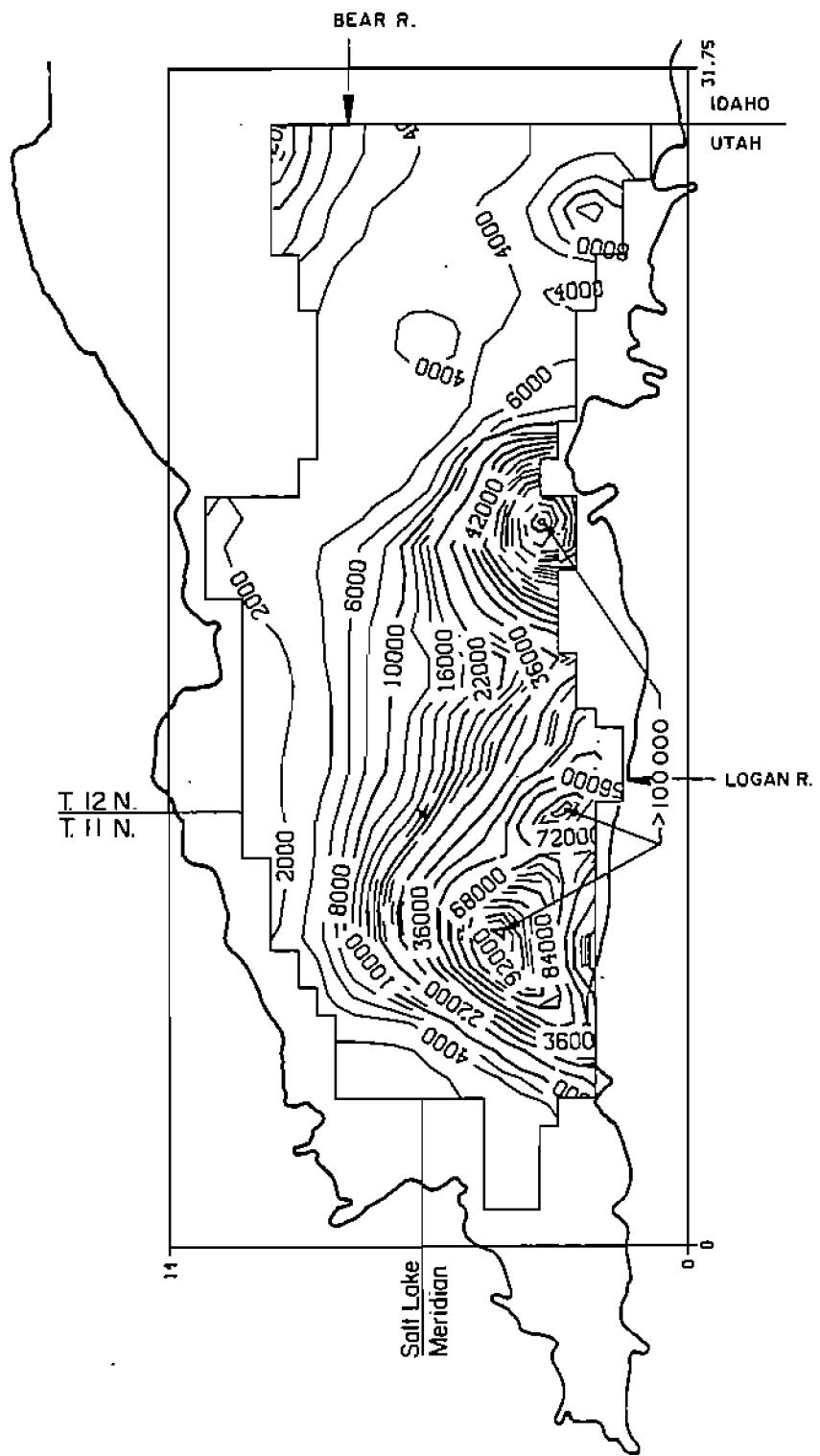


Figure 8. Transmissivity of the confined aquifer resulting from calibration of the model.

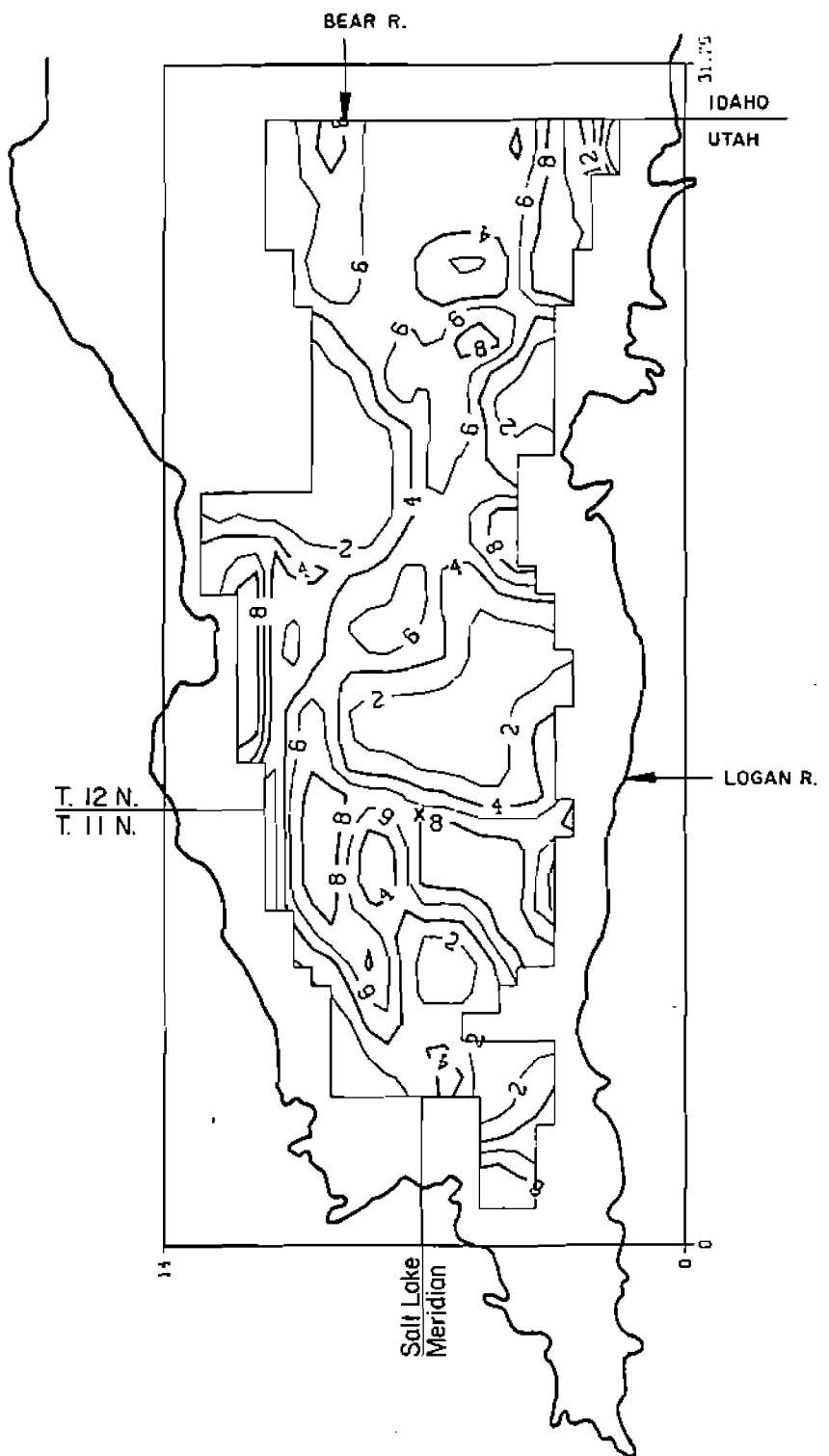


Figure 9. Hydraulic conductivity of the unconfined aquifer resulting from calibration. Contour values have been multiplied by 10,000.

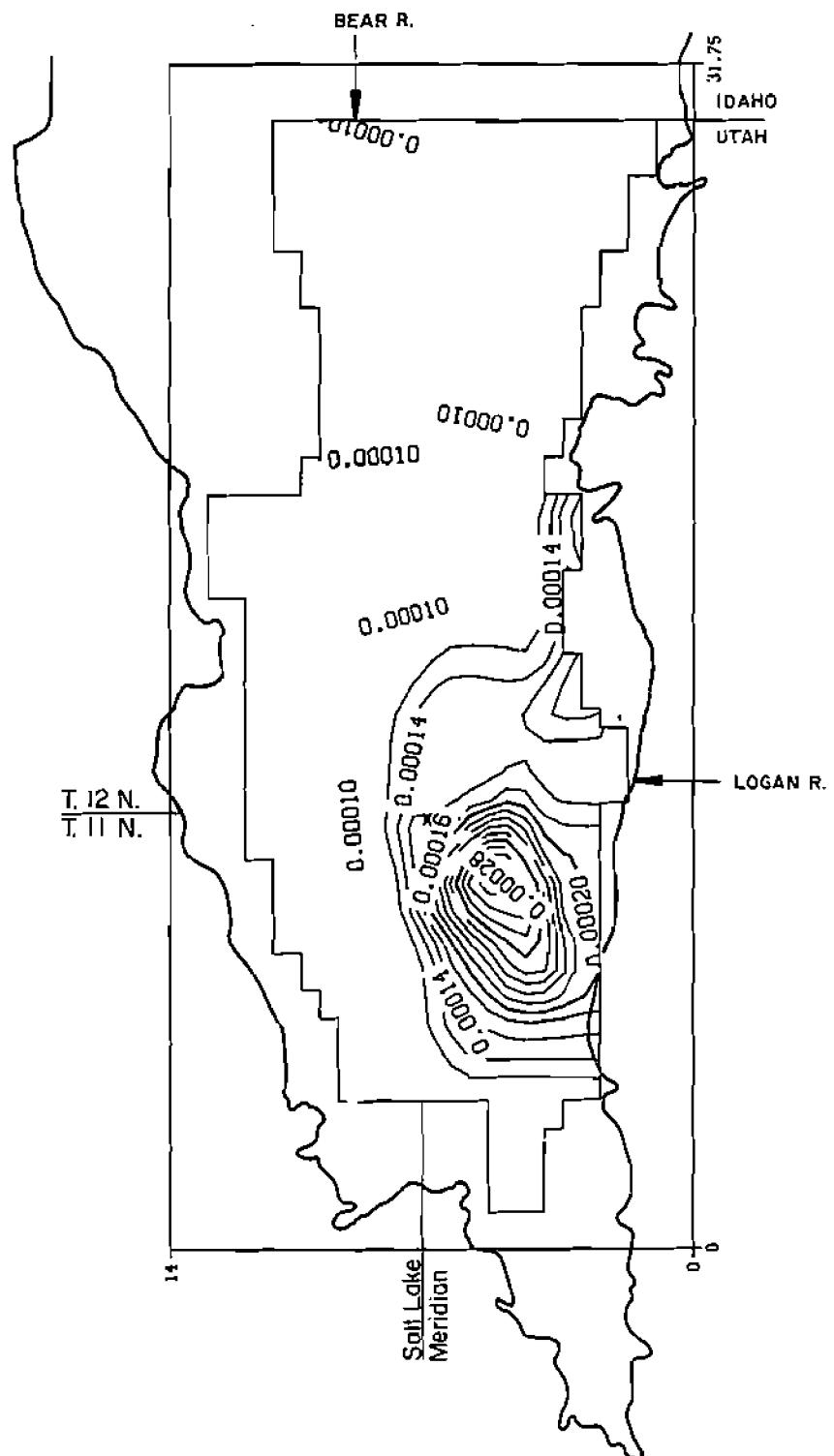


Figure 10. Storage coefficient value of the confined aquifer resulting from calibration.

where

K_{zz} = vertical hydraulic conductivity for the confining bed [L/t]

b = thickness of the confining bed [L]

z = thickness of the aquifer [L]

i, j, k = nodal point indices in x , y , and z directions

The leakance values are then equal to the ratio K_{zz}/b for each confining bed and leakance is defined as the vertical hydraulic conductivity (ft/s) per unit thickness (ft) between layers of the model.

Since few data on leakance were available for Cache Valley, Equation 5 could not be used to obtain trial leakance values. Instead the same initial value ($1 \times 10^{-10} ft/sec$ per ft) was assigned at all the nodes. Then the leakance was varied during the steady-state calibration to improve agreement between measured and calculated potentiometric surfaces. Leakance values strongly affected both the convergence to a solution and also the water level. This sensitivity of the model to the leakance values gave a good basis for adjustment of the leakance during calibration. The final values resulting from the calibration ranged from 3.5×10^{-12} to 3.5×10^{-9} feet per second per foot and are tabulated in Appendix C.

Bottom elevation of water table unit

The thickness of the water table unit ranged from 30 feet to about 200 feet depending on the different geologic units and locations (see Table 4 in Bjorklund and McGreevy 1971). The thickness in the center of Cache Valley is about 50 feet and increases toward the edges of the valley. To obtain the

bottom elevation of the water table unit, the model grid system was plotted on the 1:25,000 topographic map. The ground surface elevation for each node was determined from the contour lines. The water table aquifer thickness was estimated from Bjorklund and McGreevy (1971) based on other nearby geologic data. The bottom elevation of the water table unit for each node was then calculated from the ground surface elevation by subtracting the estimated thickness of the water table unit.

River node leakage

The total accretion from groundwater to streams is estimated to be about 140,000 acre-feet annually within the Cache Valley. Many streams or creeks originate at springs within the valley, collect additional water from springs, seeps, and drains along the way, and flow to the Bear River or one of its tributaries. The principal rivers and creeks that originate outside the valley, gain flow within the valley from springs and seeps along their courses or from tributaries that originate within the valley (Bjorklund and McGreevy 1971).

The effects of the streams on the groundwater were simulated by the method described by Prickett and Lonnquist (1971, p. 33) using Darcy's law. The method assumes that a pervious layer forms the bed of the stream and separates the stream from the aquifer. Seepage from the stream to the aquifer becomes constant when the water level in the aquifer falls below the bottom of the streambed. Under these assumptions, the rate of flow through the streambed per unit grid block area is calculated by the equation

$$QR_{\text{river}} = [K * A * (RH - PHI)/b] / \Delta X_j \Delta Y_i \\ = RC * (RH - PHI) \quad (6)$$

where

QR_{river} is the infiltration rate of the stream into the aquifer

(taken as positive when the flow is from the stream to the aquifer) [ft/sec]

K is the hydraulic conductivity of the streambed [ft/sec]

b is the thickness of the streambed layer [ft]

A is the area of the streambed within the model cell [ft²]

RH is the elevation of the river surface [ft]

PHI is the head in the top layer aquifer or the elevation of the bottom of river bed, whichever is greater [ft]

RC is the river node coefficient, representing K*A/(bΔX_jΔY_i) and varied during the calibration [l/sec]

The 75 river nodes in the model (Figure 11) are used to simulate interaction with the rivers including Bear River, Logan River, Blacksmith Fork River, Little Bear River, Spring Creek, Summit Creek, and High Creek. In these locations the infiltration rates depend on river stage and groundwater elevation as in Equation 6. River nodes only exist in areas with unconfined aquifers, and mostly occur in the central valley floor. Where streams exit from canyons and recharge the groundwater by infiltration directly into coarse beds near the mountains, conditions are better represented by constant finite flux nodes as explained earlier and summarized in Figure 5. The river channels of Bear River and Cub River upstream of their junction are treated as constant head nodes instead of river nodes in order to separate the two unconfined aquifers and to more realistically represent infiltration conditions in that area.

Spring nodes leakage

In Cache Valley, many springs, seeps, and drains discharge water from the shallow unconfined aquifers. In the flowing well areas, part of the discharge moves upward from the artesian aquifers into the shallow unconfined aquifer (Bjorklund and McGreevy 1971).

Spring nodes were assigned in the model wherever one or more relatively large spring was located within a grid. The flow rate per unit grid block of the spring node (ft³/sec) is simulated by

$$QR_{\text{spring}} = - \text{FLD} * (\text{PHI} - \text{ELD}) \quad (7)$$

where "-" sign represents water discharge from the aquifers.

FLD is a spring node coefficient varied during the calibration [ft²/sec].

PHI is the water head level of the unconfined aquifer [ft].

ELD is the altitude of the spring above mean sea level as determined from topographic maps or with a hand level where necessary [ft].

There are 21 spring nodes assigned in the model (see Figure 11). Most of the springs are assigned the spring altitude and discharge in McGreevy and Bjorklund (1970).

Evapotranspiration

The annual evapotranspiration from the 22,440 acres of wet lands in Cache Valley in Utah is estimated to be about 63,000 acre-feet (Bjorklund and McGreevy 1971), which includes evaporation from the land surface and transpiration by phreatic vegetation. It occurs mostly in the wet meadow lands in the lower parts of the valley, where the potentiometric surface of the groundwater reservoir is

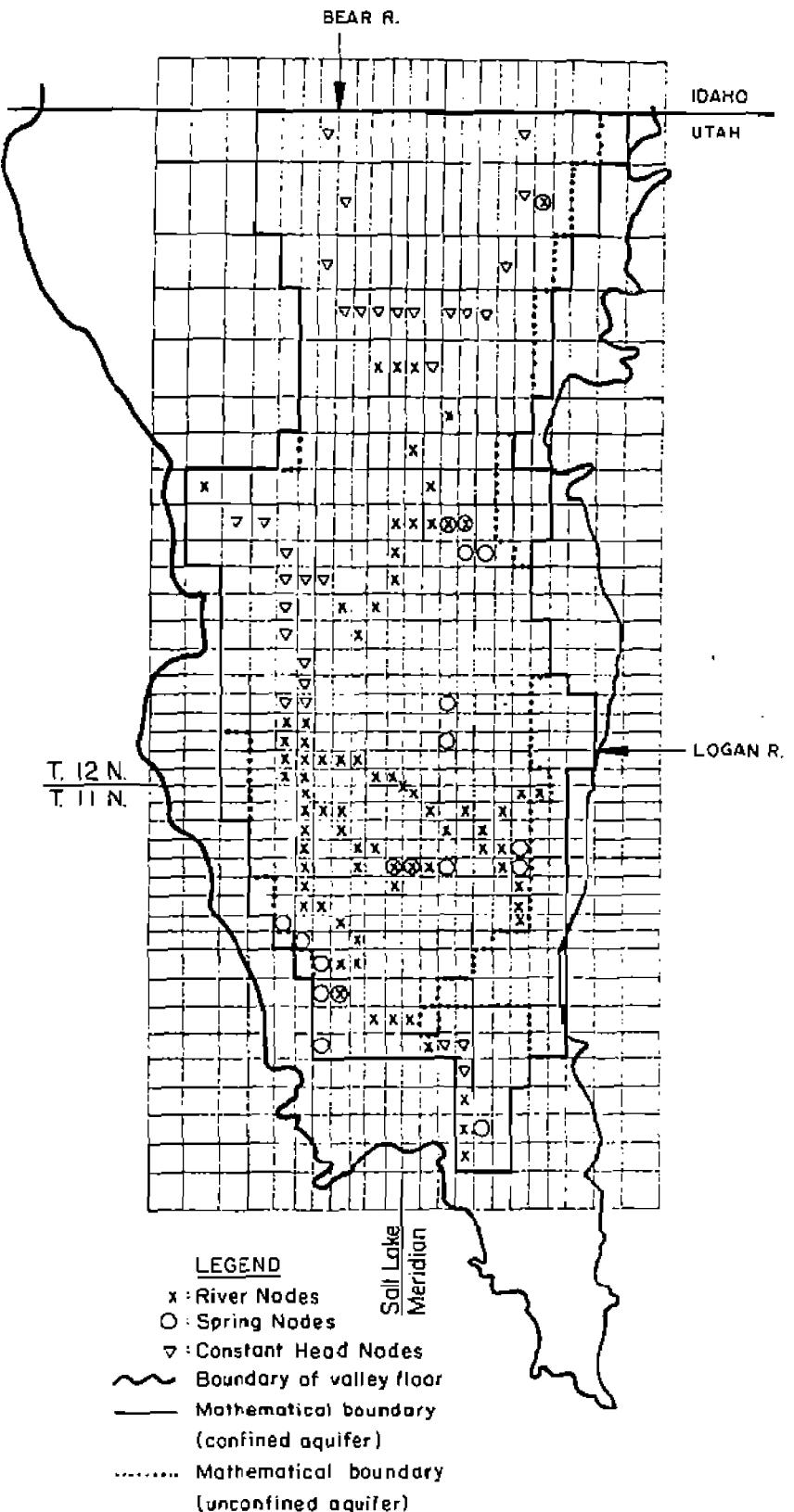


Figure 11. Location of river nodes, spring nodes, and constant head nodes.

above the land surface (Bjorklund and McGreevy 1971).

The three-dimensional computer simulation model obtained from the USGS did not include evapotranspiration. In Cache Valley, evapotranspiration is an important groundwater loss. To better match the true situation, evapotranspiration was added to the model by modification of the computer code. To avoid numerical difficulties or oscillations during convergence, evapotranspiration was treated as a linear function of depth below the land surface (Figure 12) (Trescott et al. 1976) in the relationship:

$$RET_{i,j,k} = \begin{cases} QET & \text{for } [H_{i,j,k} \geq G_{i,j}] \\ QET - \frac{QET}{ETDIST} (G_{i,j} - H_{i,j,k}) & \text{for } [ETDIST} > (G_{i,j} - H_{i,j,k}); H_{i,j,k} < G_{i,j}] \\ 0 & \text{for } [ETDIST} \leq (G_{i,j} - H_{i,j,k})] \end{cases} \quad (8)$$

where

$RET_{i,j,k}$ = evapotranspiration rate [$\text{ft}^3/\text{sec per ft}^2$]

QET = maximum evapotranspiration rate [$\text{ft}^3/\text{sec per ft}^2$]

$ETDIST$ = depth below ground surface at which evapotranspiration ceases [ft]

$G_{i,j}$ = ground surface elevation at node i,j [ft]

$H_{i,j,k}$ = water level elevation at node i,j,k

Pumpage

At present, there are totally about 2,950 wells, including both flowing and pumping wells, in Cache Valley within Utah (Logan District Office

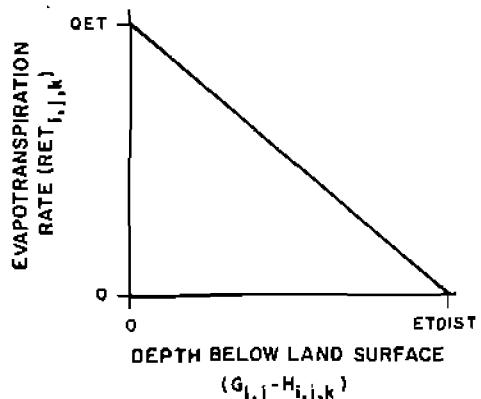


Figure 12. The linear relationship between the evapotranspiration and the depth below the land surface.

of the Utah Water Rights Division). Detailed information on discharge, pumping duration, and drilling logs are not available for many of the wells, especially for smaller private wells. Without discharge data, it is impossible to sum the discharge from the wells within a grid cell for a node total. An approximate method for estimating well discharge by node was substituted.

First, all the discharge wells in Cache Valley were classified as "large" or "small" wells according to whether the flow rate was greater than 350 gallons per minute. The large wells were identified and located individually on the grid system map. The number of small wells in each grid cell (node) is tabulated in Figure 13. An annual mean discharge of small wells was estimated by subtracting from the total annual pumpage reported in "Groundwater Conditions in Utah" the

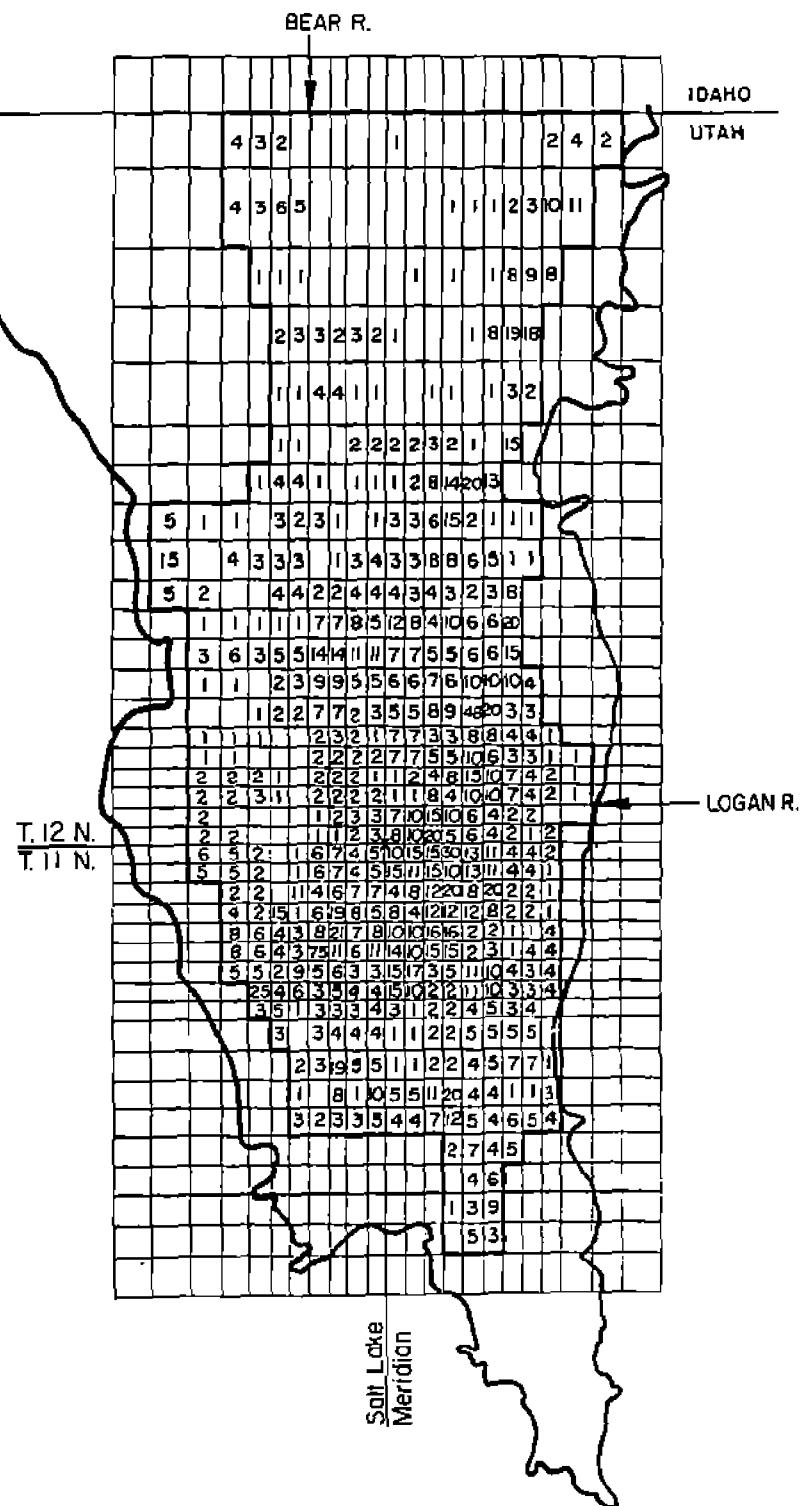


Figure 13. Total number of "small" wells within each grid cell.

annual discharge from all the large wells and then dividing by the total number of small wells. The total pumpage for each cell was then taken as the product of mean small-well discharge, times the number of small wells, plus the discharge of the large wells within the cell.

Most large irrigation and municipal wells are pumped at full capacity for 3 months or less, usually starting when surface water supplies diminish in mid June, through August or September. Some of the large wells are used only in dry years. A detailed review of electric power use records would be one way to reconstruct the pumping history of large wells. From a personal communication

with the Preston Office of the Utah Power and Light Company, it was learned that existing power company records summarize the total power usage from the pumping of both groundwater from well and surface water from rivers and canals. There is no way to identify groundwater pumping power separately except by detailed analysis of old records.

Because the resources for such a study were not available, an alternative method was used to estimate larger well pumpage. Guided by the pumping histories of a few typical large wells, it is assumed that the large wells are pumped at rated flow rate during 4 hours each day for 5 months of the years.

MODEL CALIBRATION

The purpose of calibration is to match observed groundwater conditions with the simulation as closely as possible by adjusting the model parameters within hydrologically reasonable limits. The model is calibrated by repetitively running the computer program using available hydrogeologic data, noting differences between simulated and recorded conditions and varying selected model parameters to improve the match. If the model can be calibrated to reproduce historical field measurements, then one has greater confidence in its ability to predict what will happen to the groundwater under various assumed conditions in the future.

Developing the Cache Valley model involved both a steady-state calibration and a transient-state calibration. First, under steady-state conditions starting from assumed March 1969 heads, the model parameters were adjusted so that the iterative solution closely matched the potentiometric surface of March 1969 in Figures 6 and 7. The transient-state calibration includes two steps: first, a simulation to reproduce the potentiometric surface of March 1969 after adding the non-zero storage coefficient values to the parameters determined from the steady-state calibration; and second, simulations under transient conditions to match year by year the annual water level change maps from 1969 through 1972. Besides the agreement between the calculated and historical water levels, another check on the accuracy of the simulation was to compare computed recharge and discharge values with the historical water budget analysis for each pumping period.

Steady-State Calibration

Groundwater levels in the main aquifer in Cache Valley in Utah have not changed significantly since 1935, although seasonal fluctuations occur (Bjorklund and McGreevy 1971). The valley is an overflowing groundwater basin, where water levels are stabilized. The hydrographs in plate 2 of Bjorklund and McGreevy (1971) also indicate that the natural recharge-discharge relationship has not been changed much by the withdrawal of water from wells.

The steady-state simulation was calibrated against the water level maps of March 1969 and using these same water levels as the initial values in the computation. The aquifers were not truly in a steady-state condition during 1969, but the assumption of the steady-state conditions is quite reasonable as water-level changes were small and local in nature during 1969.

During the steady-state calibration, a storage coefficient of zero was set at every node, thereby eliminating the time dependent term in Equation 1. After a series of steady-state calibrations were done, the sensitivity of the model to variations in the hydraulic parameters was better understood than before the calibration. In the Cache Valley model, the first and most important parameter to be adjusted was the vertical hydraulic conductivity or leakance, because of its important role in the iteration convergence in the model. This sensitivity is easy to understand because upward leakage to the water-table aquifer is the principal means of natural discharge

from the artesian aquifer over 200 square miles of the Cache Valley area. The leakage could not be measured directly, so leakance values were varied at many nodes during calibrations in order to balance the total recharge and discharge. The general calibration procedure for adjusting leakance values proceeded by steps. First, a multiplication factor of leakance value common to all nodes ("FAC", see Appendix B) was adjusted until the maximum difference between computed and observed values became small. Then more detailed individual variation for each node was done to improve the convergence and further reduce the error differences.

The aquifer transmissivity and horizontal hydraulic conductivity also strongly affect the model. These parameters are adjusted according to the following guidelines: If the calculated water levels in an area are lower than observed water levels, then the transmissivity and hydraulic conductivity values at nearby nodes should be decreased; but if the calculated water levels are higher, larger values should be used. Other parameters such as river node coefficients, spring node coefficients and the recharge flow across the boundaries were also adjusted. The maximum evapotranspiration rate is a sensitive parameter and is varied during the calibration by comparing the computed value to the reported 63,000 acre-feet of total annual evapotranspiration in Cache Valley in Utah. It is assumed that 1969 is an "average" evapotranspiration year.

The computed steady-state water level maps are shown in Figures 14 and 15 for the confined and unconfined aquifers respectively. The comparison between computed and observed water levels after steady-state calibration showed the maximum absolute difference in the confined aquifer was about 7 feet and in the unconfined aquifer was about 9 feet. The mean absolute difference was about 1.8 feet in the confined aquifer and about 2.3 feet in the

unconfined aquifer. The map showing the computed differences between the steady-state calibrated water levels and the observed water levels is given in Appendix E. Along the edge of the basin, steep groundwater gradients sometimes caused difficulty in matching water levels.

Transient-State Calibration

The transient-state calibrations began by using the observed water level conditions of March 1969 as initial values as shown in Figures 6 and 7. The parameter values resulting from the steady-state calibration were used as starting values along with the assumed non-zero storage coefficients. Parameters were adjusted in successive simulations to reproduce the March 1969 conditions. The calculated water level conditions of March 1969, after transient-state calibration adjustments are shown in Figures 16 and 17. The maximum absolute difference value between the computed and the initial observed water level was about 8 feet in the confined aquifer and about 8 feet in the unconfined aquifer. The mean absolute difference value was about 1.66 feet in the confined aquifer and 1.64 feet in the unconfined aquifer. Comparing with the mean difference values of 1.79 feet and 2.30 feet from the steady-state calibration, the overall water level agreement for the condition of March 1969 was somewhat improved by the transient-state calibration.

The next step in transient-state calibration started from the calculated water level condition of March 1969 and simulated the effects of three successive one year periods. Model parameters were adjusted to achieve the best match year by year with water level change maps until March 1972. Those maps shown in Figures 18a, 19a, and 20a were taken from the water level change maps as shown in the annual reports on "Groundwater Conditions in Utah." Except for the parameters whose values do physically change from year to year (the storage

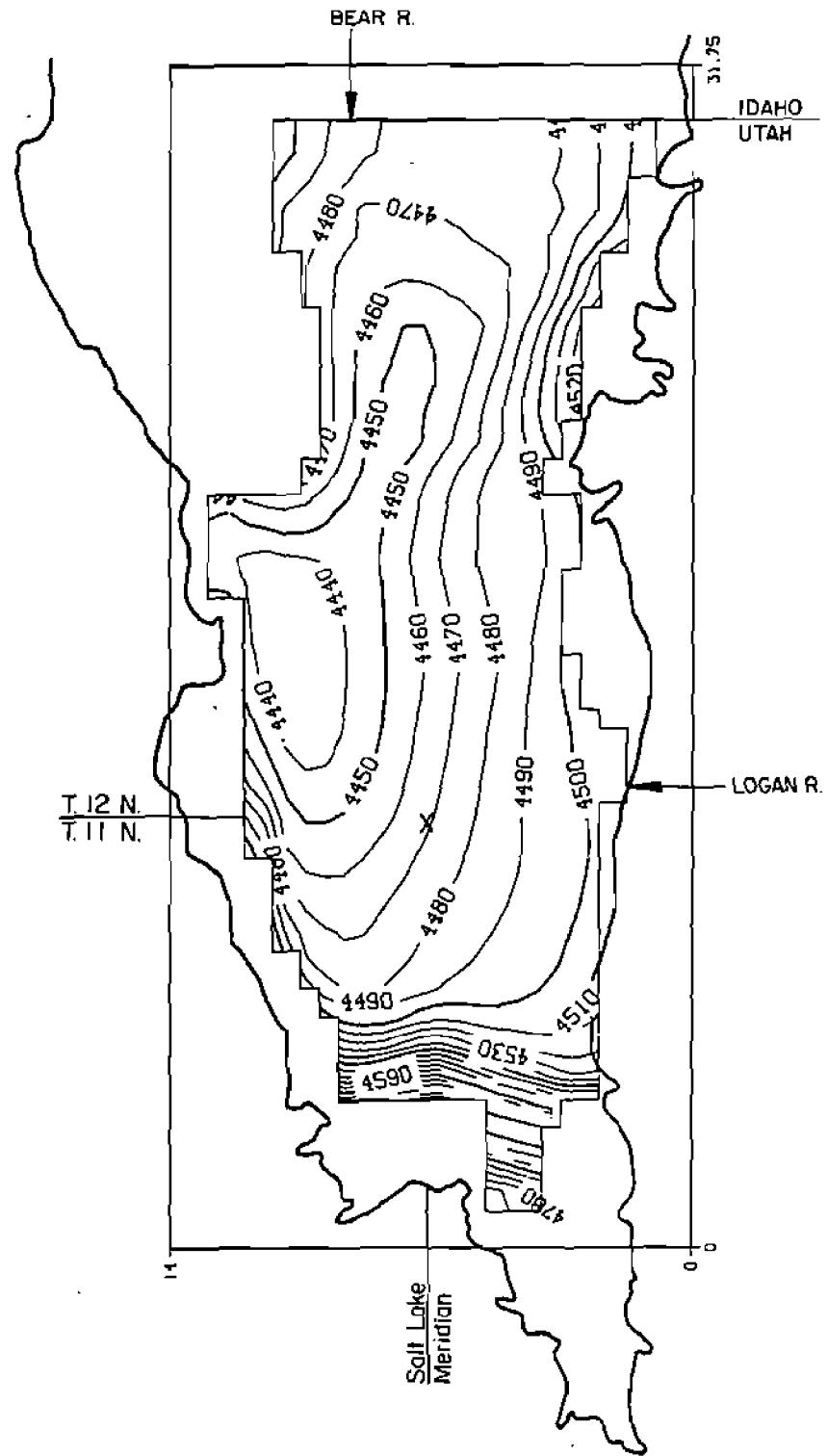


Figure 14. Calculated steady-state water level contour map of confined aquifer.

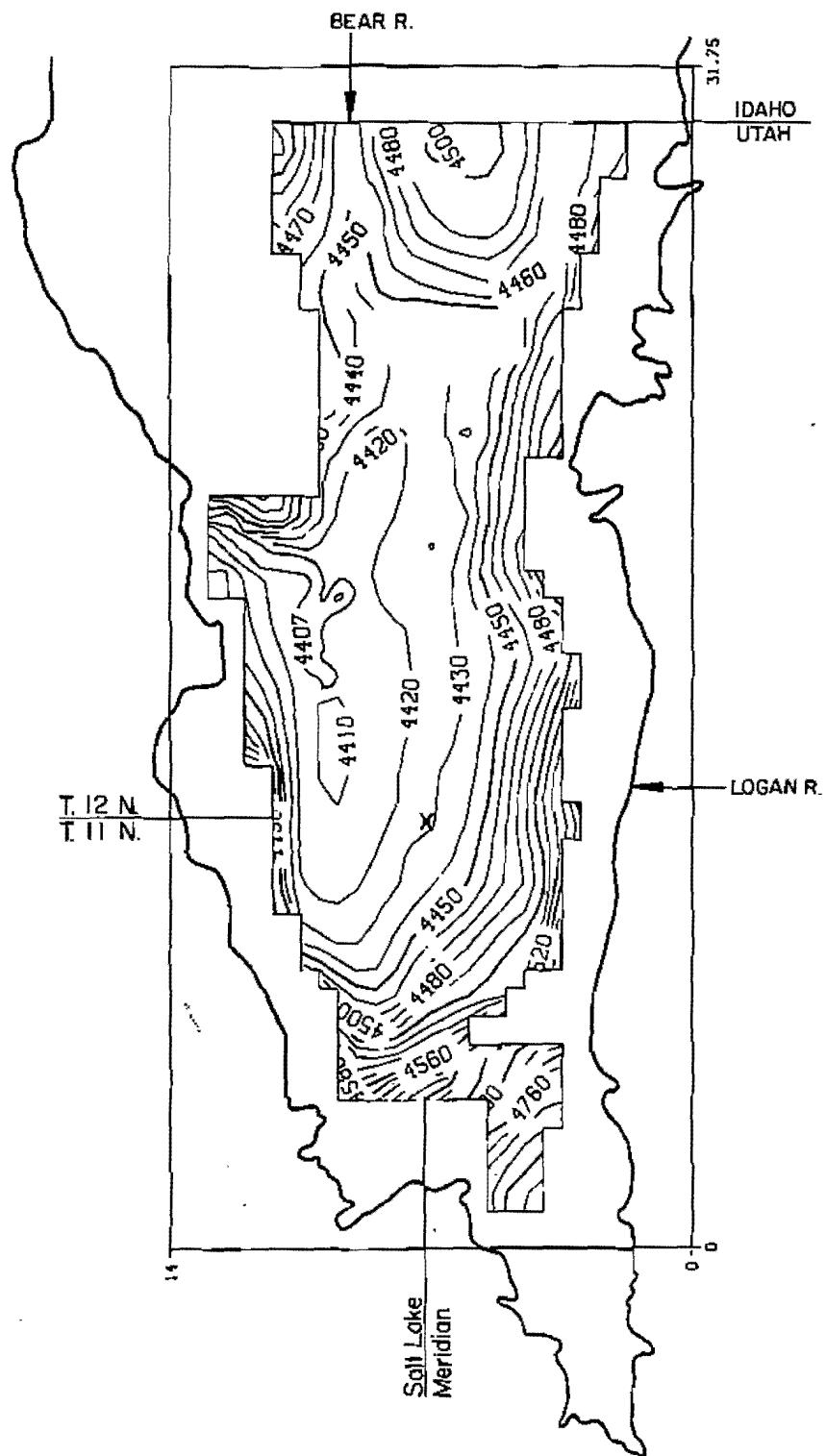


Figure 15. Calculated steady-state water level contour map of unconfined aquifer.

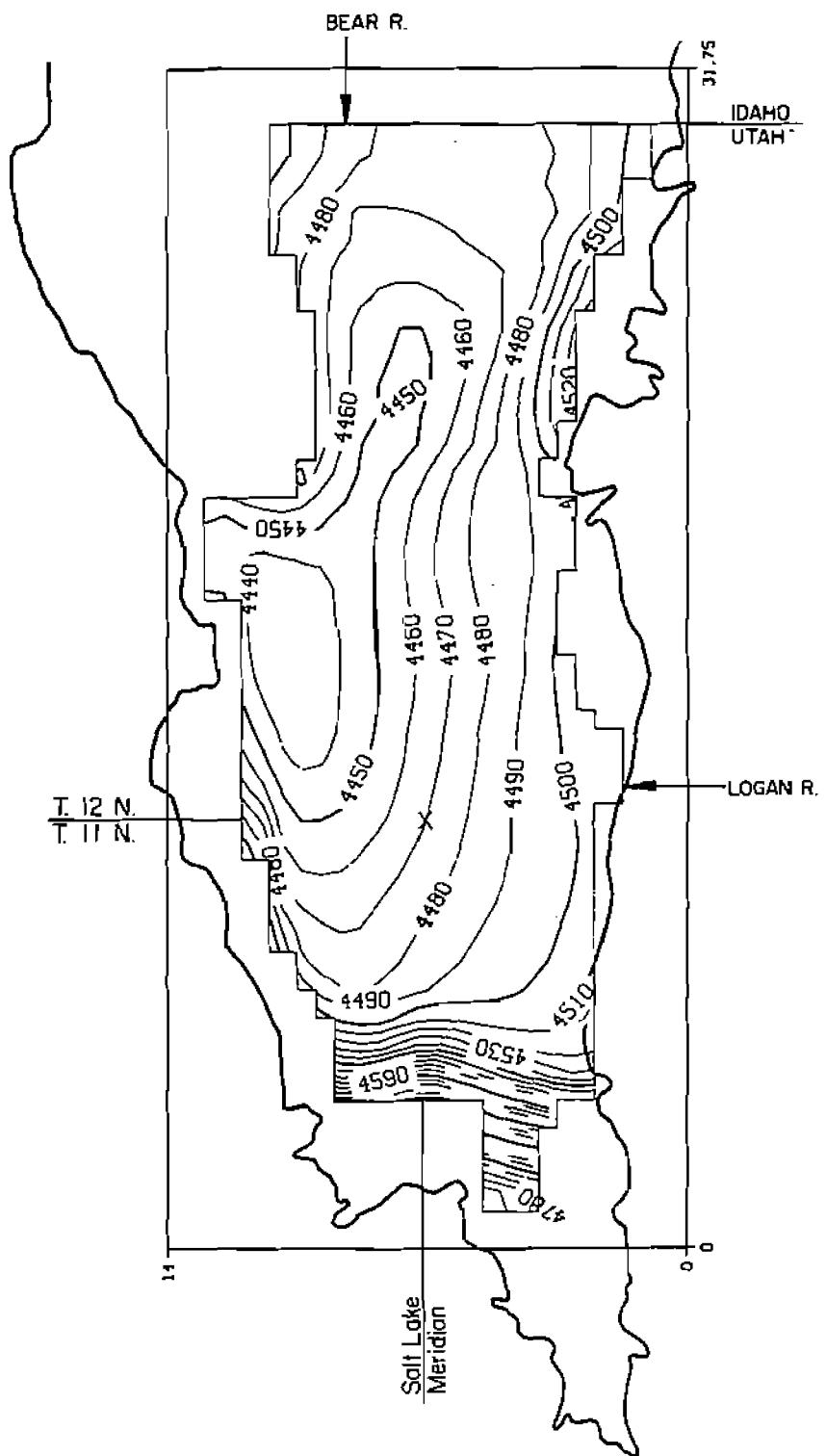


Figure 16. Calculated water level contour map of confined aquifer of March 1969.

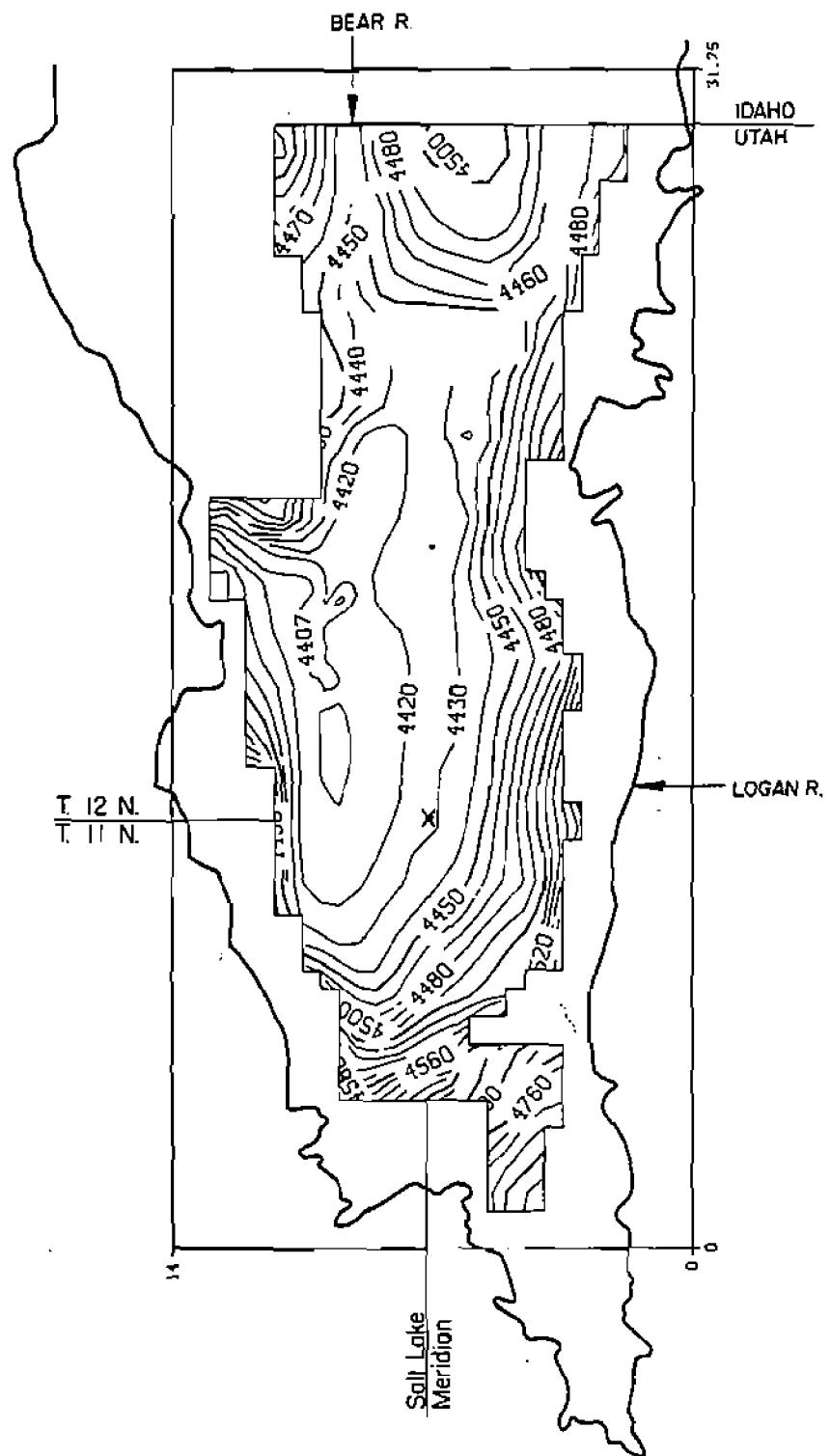
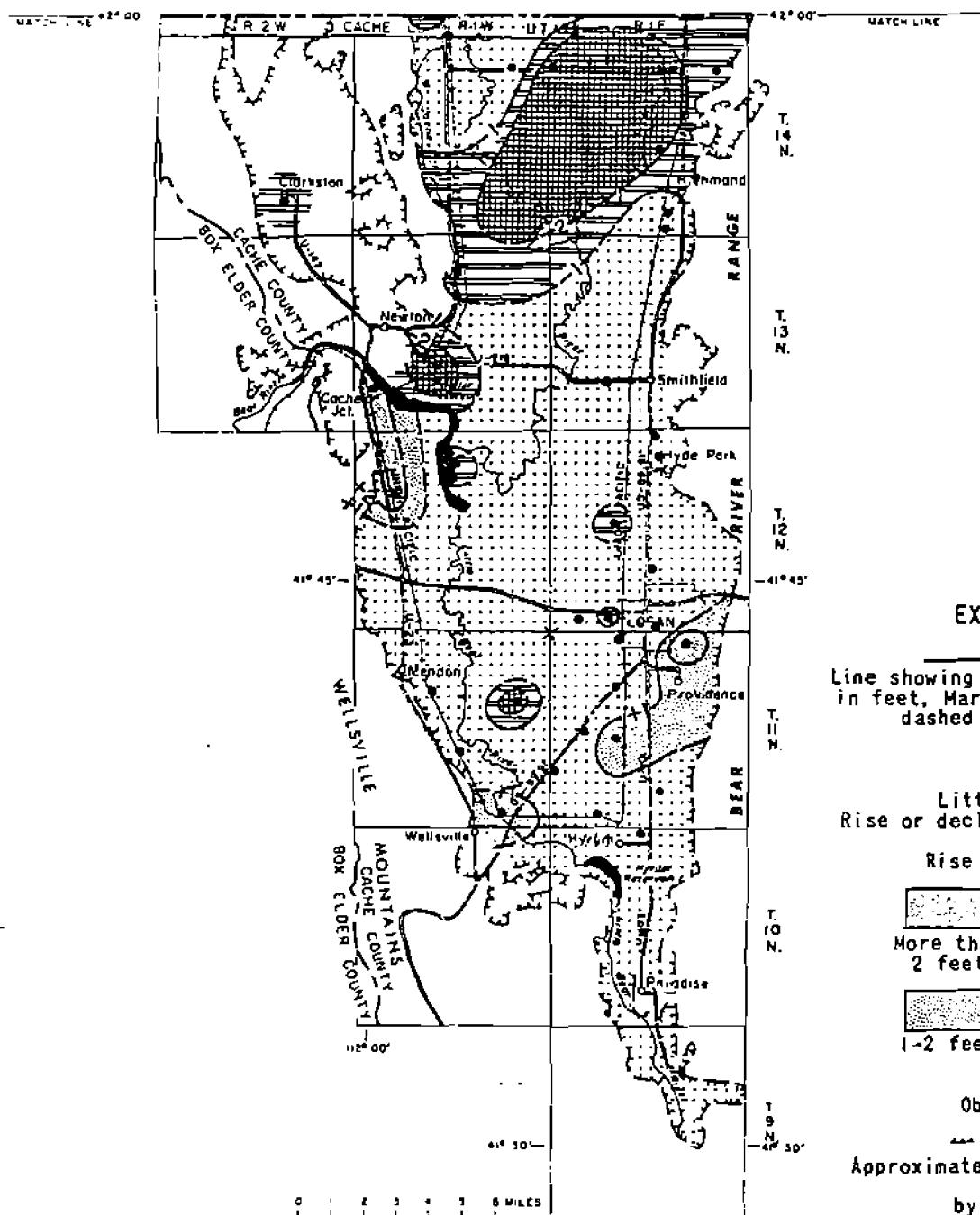


Figure 17. Calculated water level contour map of unconfined aquifer of March 1969.



EXPLANATION

Line showing change of water level,
in feet, March 1969 to March 1970:
dashed where approximate

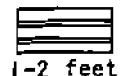


Little or no change
Rise or decline of less than 1 foot

Rise



Decline



More than
2 feet

1-2 feet

1-2 feet

More than
2 feet

Observation well

Approximate boundary of valley fill

by L. J. Bjorklund

Figure 18a. Observed water level change map of confined aquifer from March 1969 to March 1970. (From "Groundwater Conditions in Utah" 1970.)

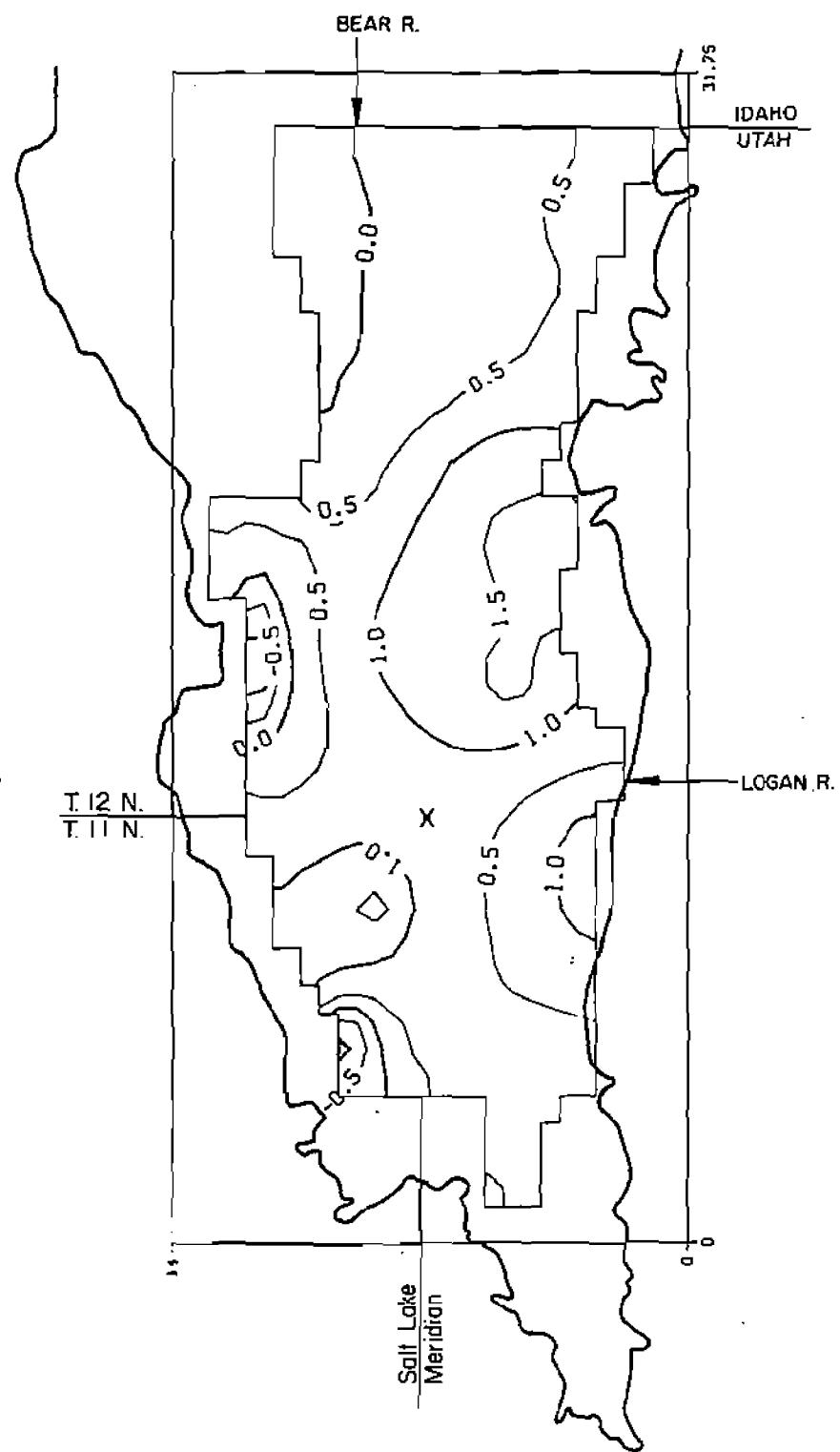


Figure 18b. Calculated water level change map of confined aquifer from March 1969 to March 1970.

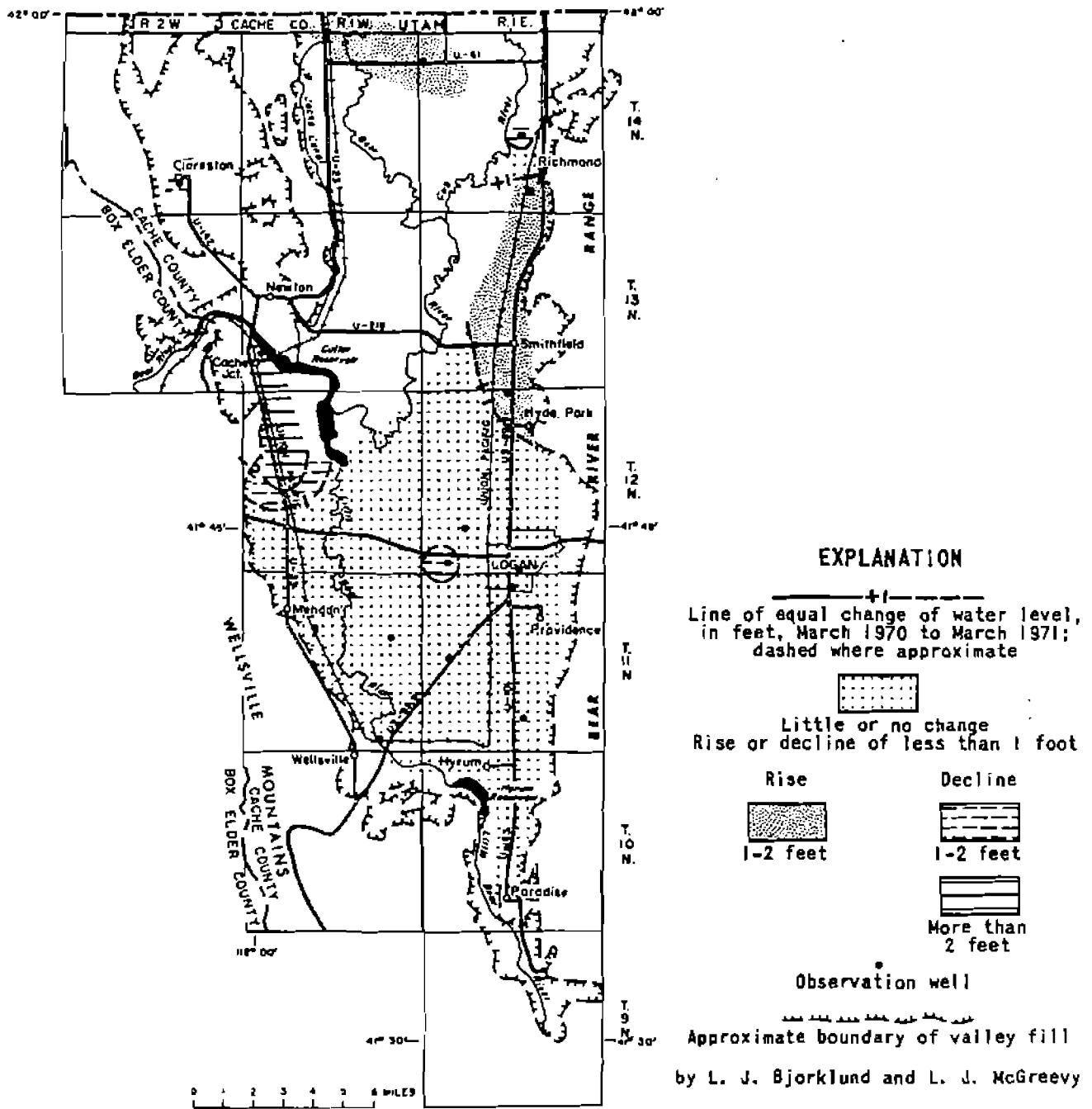


Figure 19a. Observed water level change map of confined aquifer from March 1970 to March 1971. (From "Groundwater Conditions in Utah" 1971.)

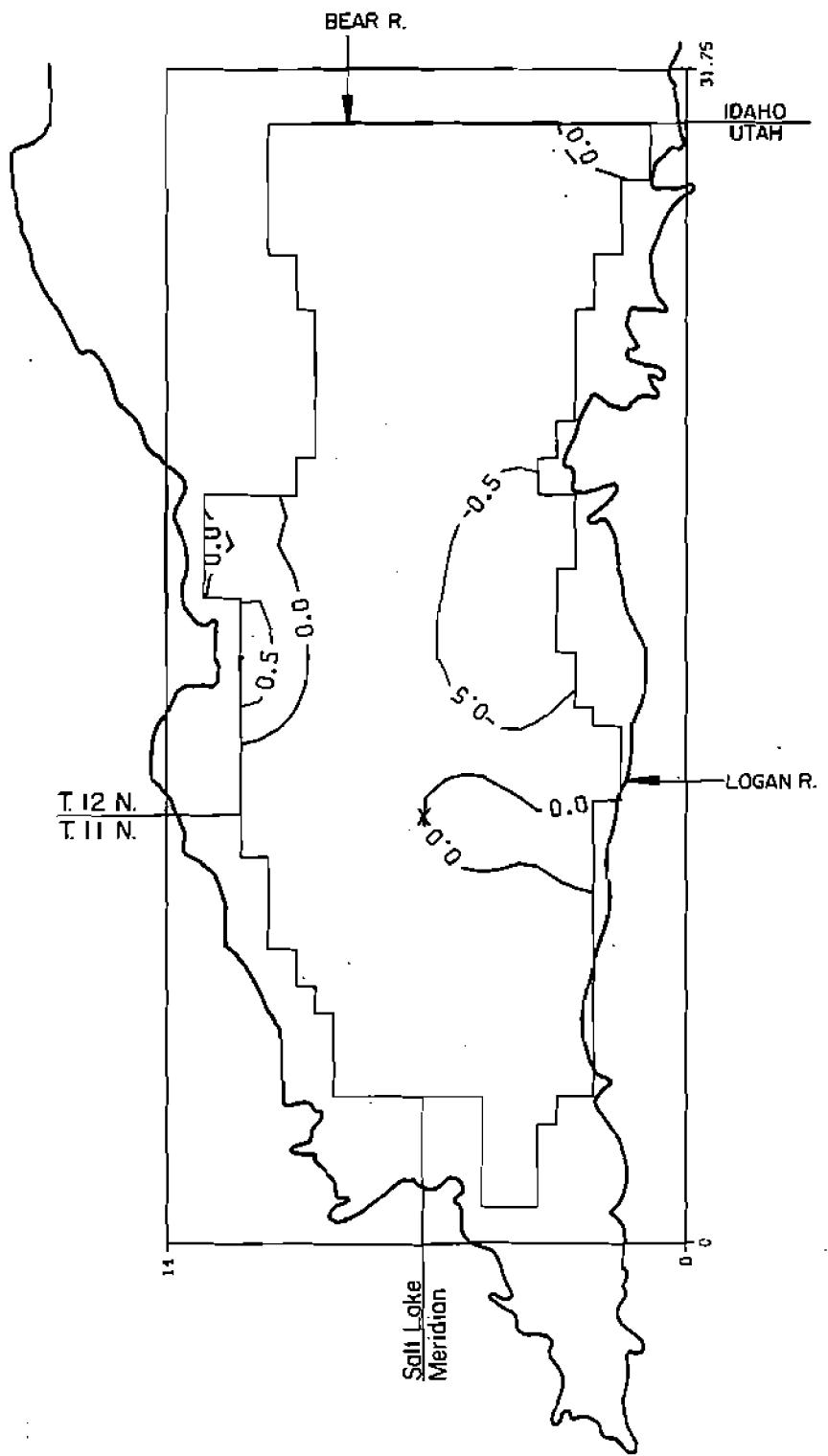
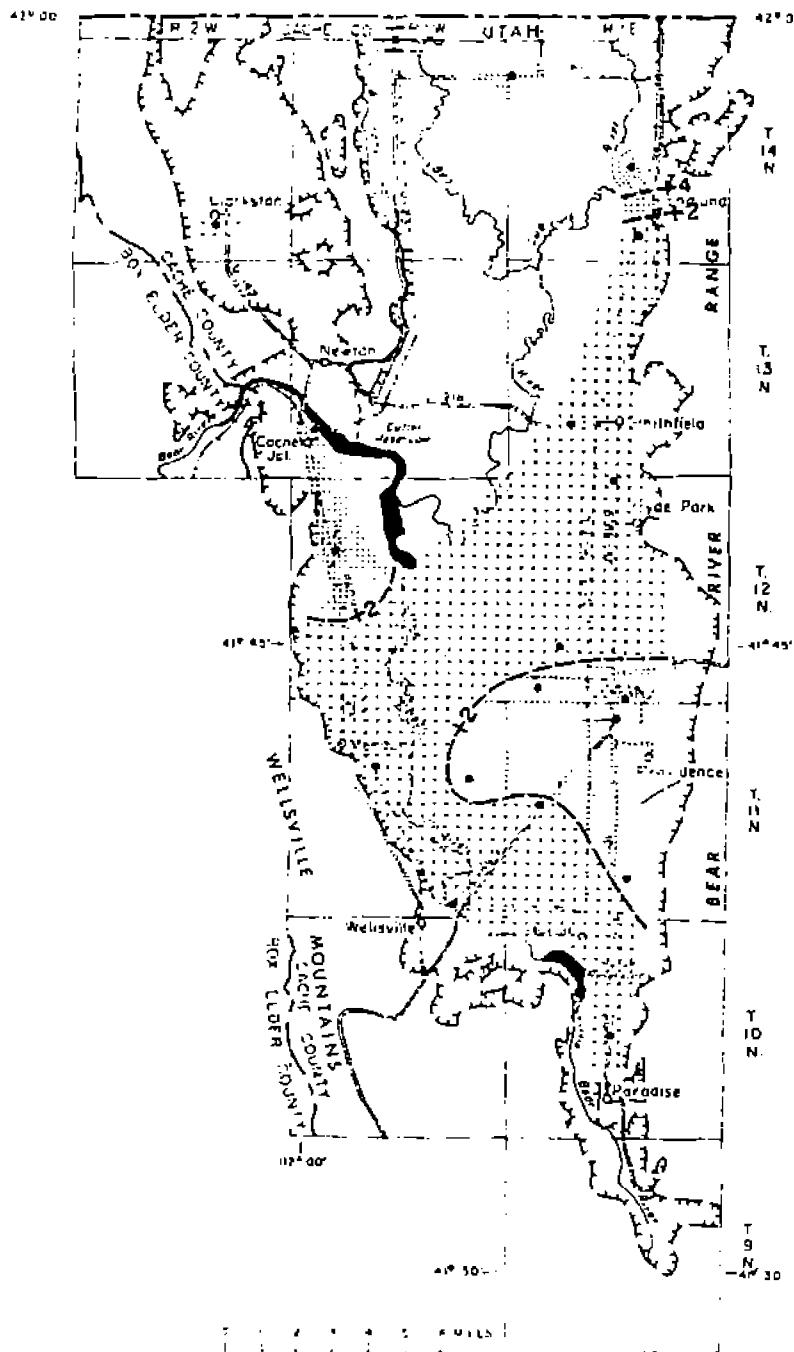


Figure 19b. Calculated water level change map of confined aquifer from March 1970 to March 1971.



EXPLANATION

-----+2-----
Line of approximate equal change of
water level, in feet, March 1971 to
March 1972

Rise	Decline
More than 4 feet	0-1 foot
2-4 feet	
0-2 feet	

* Observation well

Approximate boundary of valley fill
by L. J. Bjorklund and L. J. McGreevy

Figure 20a. Observed water level change map of confined aquifer from March 1971 to March 1972. (From "Groundwater Conditions in Utah" 1972.)

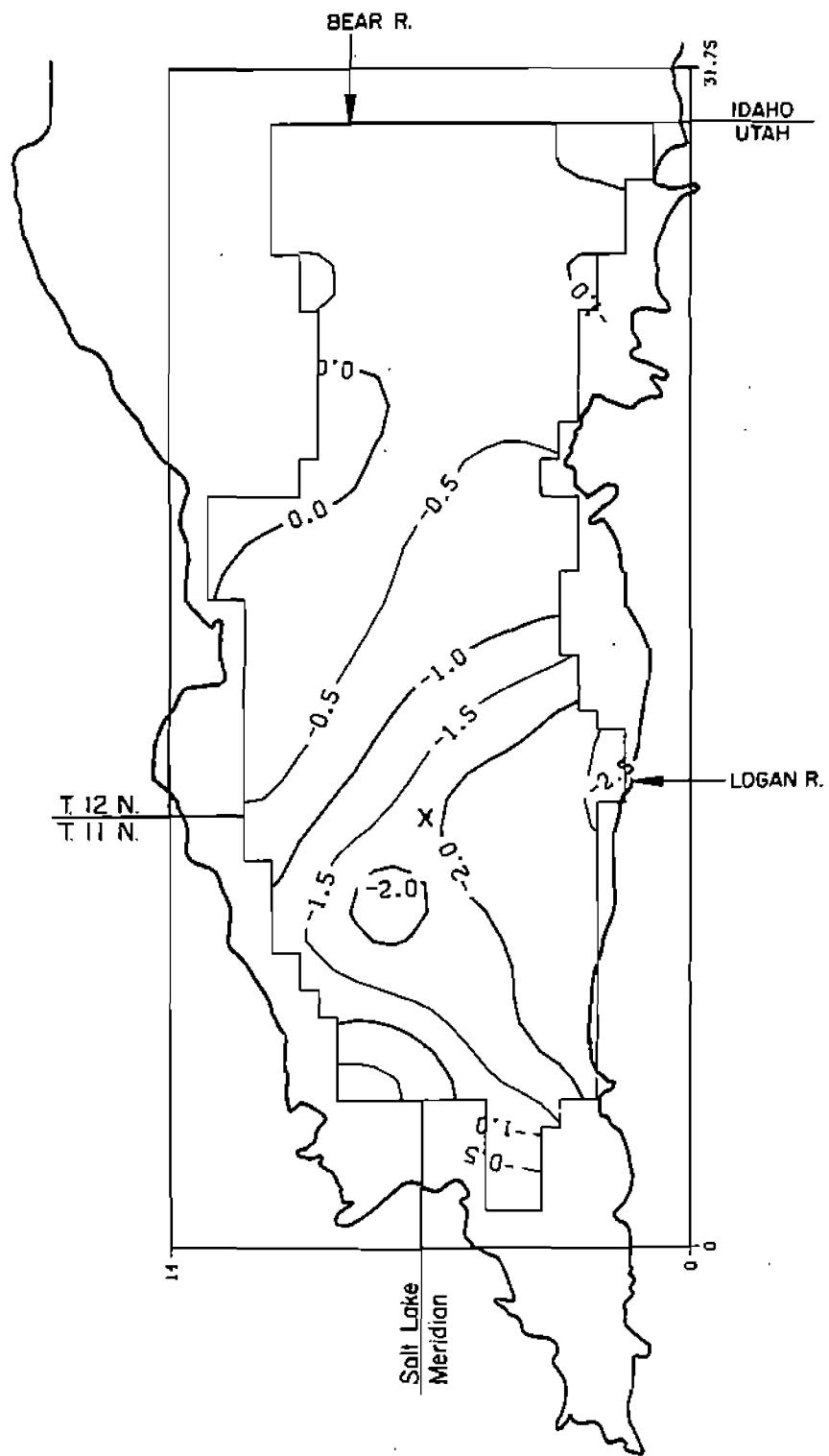


Figure 20b. Calculated water level change map of confined aquifer from March 1971 to March 1972.

coefficients, total pumpage, pumping pattern and recharge flux for each pumping period), the parameter values were not changed during the transient-state calibrations. The parameter values, primarily pumping pattern and recharge flux, were then adjusted until computed water level changes matched observed changes as closely as possible. The calculated water level change maps for the confined aquifer for each of the 3 years are shown in Figures 18b, 19b, and 20b.

The transient-state calibration was started from the calculated water levels rather than the observed for two reasons: first, the observed values were already in the solution ready for the next step; and second, starting from calculated values accumulates errors in the computation and is thus a more difficult test of the model. The model was not sensitive to variation in storage coefficients. A sensitivity study showed that storage coefficients up to 100 times larger or smaller than those used in other simulations did not seriously change model predictions.

The comparison between observed and computed water-level changes for some observation wells over a few years are shown in Figure 21. The model does respond well to different pumpage rates, different pumping patterns and different recharge for each pumping period. While the total amount of annual discharge from the wells was quite well known, other information such as pumping patterns and recharge for each year were not available and had to be assumed first and then adjusted in the calibration process.

The transient-state model parameter input data for generating the water level conditions of March 1969 and the resulting computer output are listed in Appendices C and D.

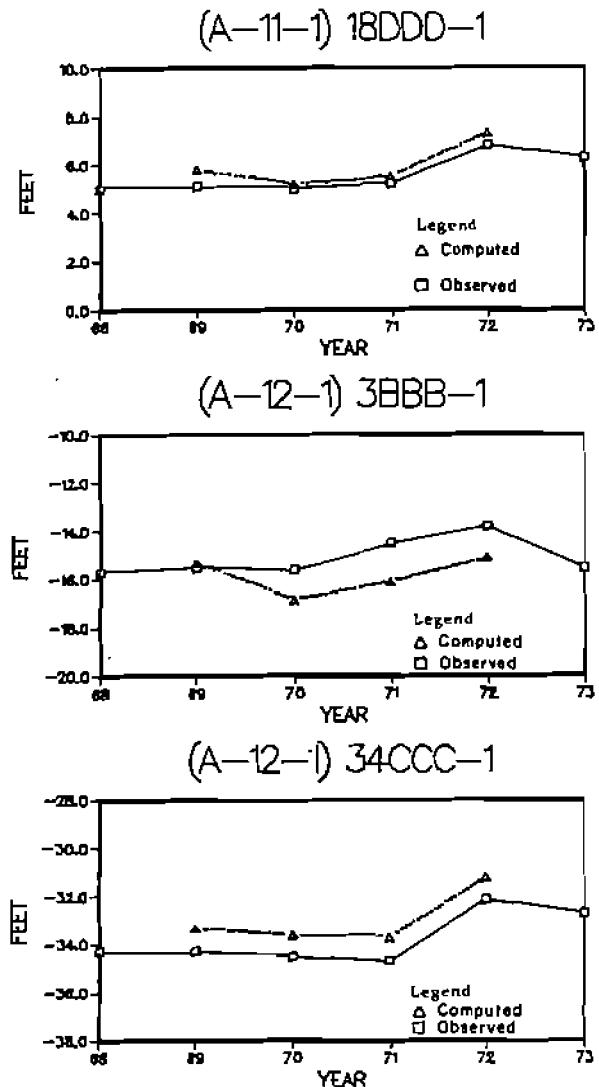


Figure 21a. Comparison between computed and observed water level changes in selected wells.

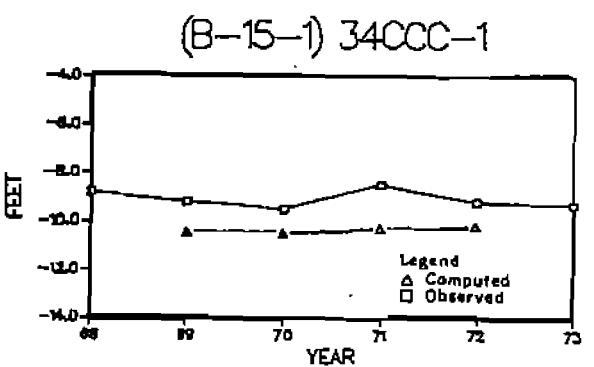
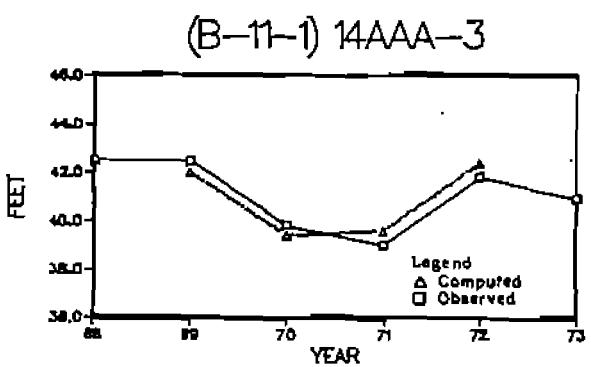
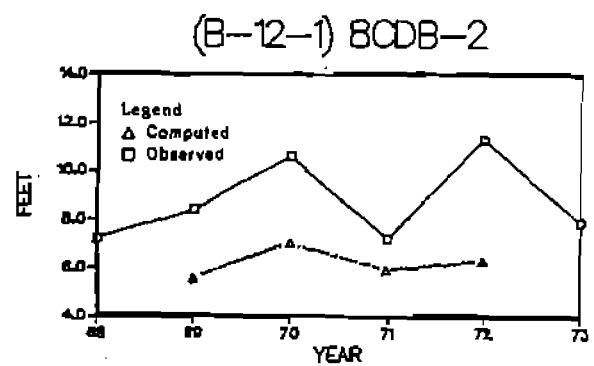
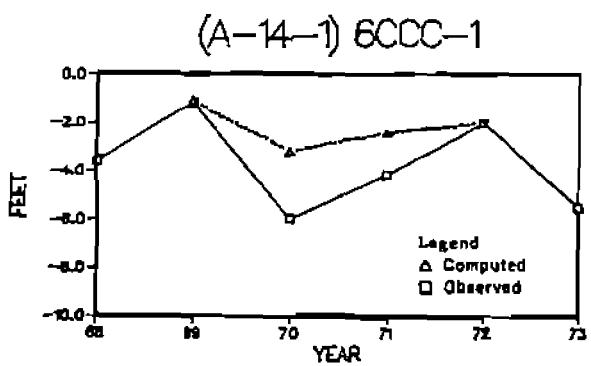
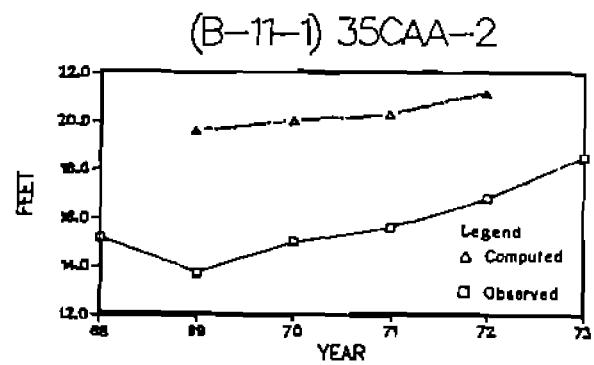
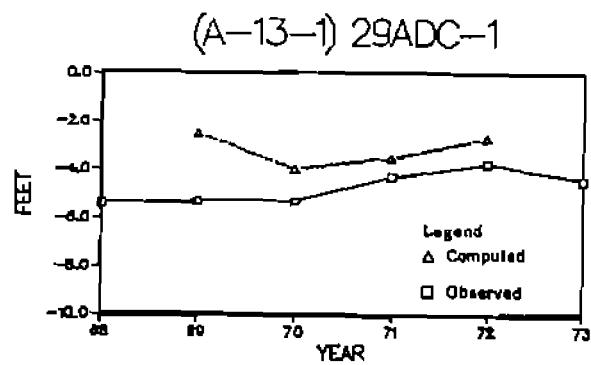


Figure 21b. Comparison between computed and observed water level changes in selected wells.

Figure 21c. Comparison between computed and observed water level changes in selected wells.

USE OF THE MODEL FOR MANAGEMENT PURPOSES

Three predictive simulations were made to examine the effects of alternative levels of groundwater development. All three began from the calculated water level configuration of March 1969. The boundary conditions and initial conditions were not changed except for the three levels of development represented by the sets of wells shown in Table 2. All the new wells were assumed to pump water entirely from the confined aquifer.

The first management study examined the effect of drilling two wells, one located at the mouth of Smithfield Canyon (at 18,30) with pumping rate of 3000 gallon per minute (gpm) and another located next to the Natural Resources Building on the campus of Utah State University (at 20,21) with 4500 gpm. Both wells were arbitrarily assumed to pump for 4 hours a day for 5 months of the one year simulation.

The next higher level of new development assumed eight additional irrigation wells, each with a flow rate of 2250 gpm and the same pumping schedule as before. The locations for the wells are shown in Table 2 and Figure 4.

The third predictive simulation assumed the same wells as the second simulation, but the flow rates were twice as large for all wells except the well at node (18,30) where the flow was four times as large. The additional wells and pumping rates in the simulations are somewhat arbitrarily chosen to represent what might happen if such development took place.

The Cache Valley groundwater model computes annual changes in water level

Table 2. New well locations and pumping rates for three predictive simulations.

Predictive Simulation	Well location (node number)	Maximum Pumping rate (gpm)
1*	(20,21) (18,30)	4500 3000
2	(20,21) (18,30) (8,13) (8,14) (9,13) (9,14) (8,26) (8,27) (9,26) (9,27)	4500 3000 2250 2250 2250 2250 2250 2250 2250 2250
3	(20,21) (18,30) (8,13) (8,14) (9,13) (9,14) (8,26) (8,27) (9,27) (9,26)	9000 12000 4500 4500 4500 4500 4500 4500 4500 4500

*Locations and pumping rates of the wells in simulation 1 were provided by the Logan Office, Utah Division of Water Rights.

in one increment for the year. Irrigation wells may pump only during the irrigation season. Municipal wells may pump more or less continuously. The model converts the pumping rate into a uniform rate per year. For example, if a well is pumping 4 hours each day for the 5 irrigation months at a flow rate of 10 cfs or 4500 gpm, when converted into a uniform rate per year, the rate will be 0.685 cfs for the entire year.

The water level drawdown maps for the confined aquifer caused by the additional wells under the three year-long management simulations are shown in Figures 22, 23, and 24. Comparison of these maps with the observed and computed water level maps, Figures 6, 7, 14, 15, 16, and 17, shows how the Cache Valley model can be used to estimate the response of the groundwater basin to various development schemes. The Cache Valley model is available as a

tool to assist in the better understanding and management of the groundwater basin.

Those wishing to apply the model in solving Cache Valley groundwater management problems should become generally familiar with the program documentation provided by Trescott (1975) and Trescott and Larson (1976). Moreover, the appendices of this report are especially valuable for those wishing to use the Cache Valley model.

Appendix A gives a complete listing of the computer program. Appendix B describes in detail all the necessary input data and their preparation. Appendix C lists the model input for the Cache Valley model and contains the parameter conditions. Appendix D presents the Cache Valley model computer output for the transient-state calibration of March 1969.

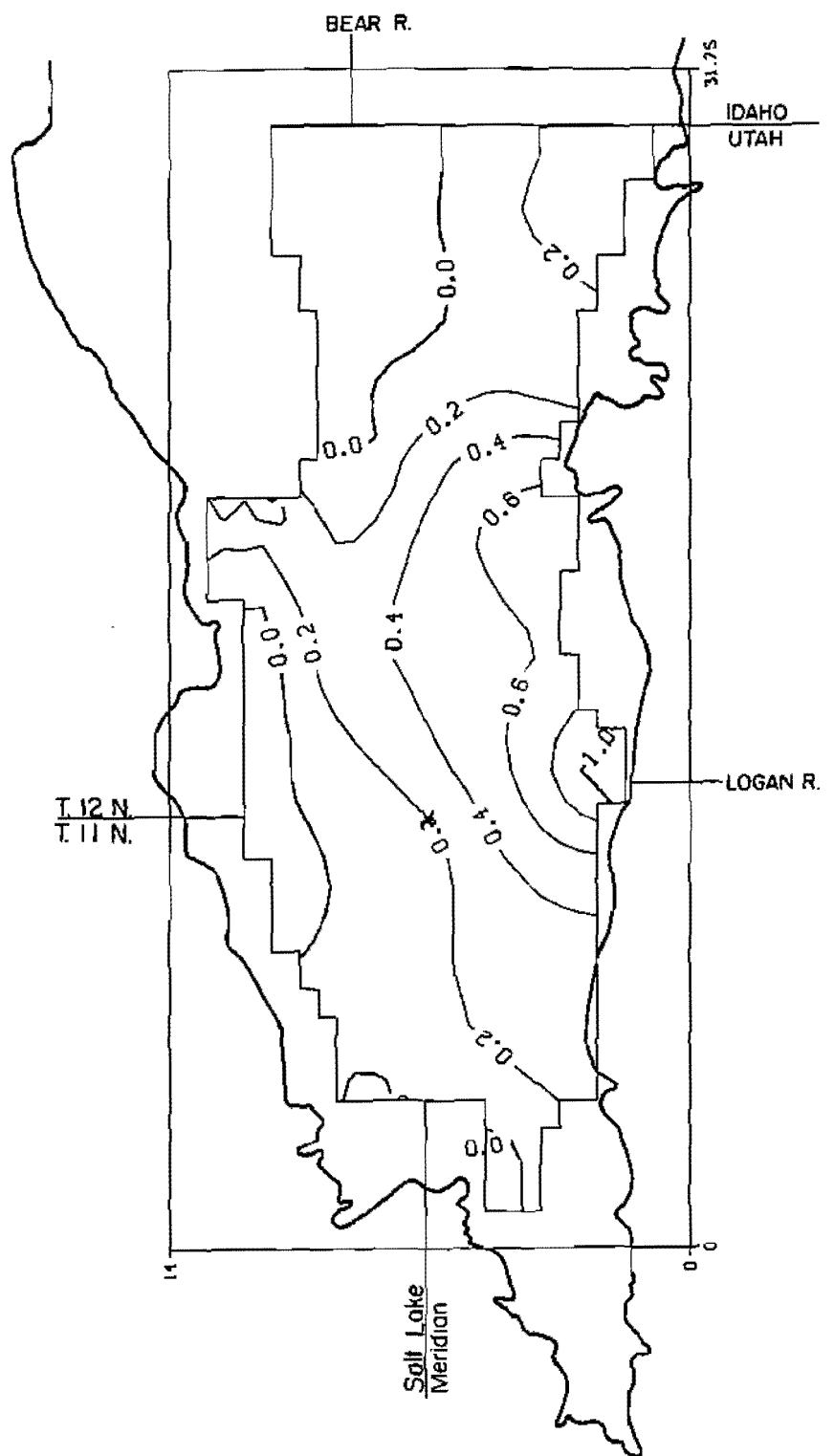


Figure 22. Water level change map of confined aquifer of predictive simulation 1.

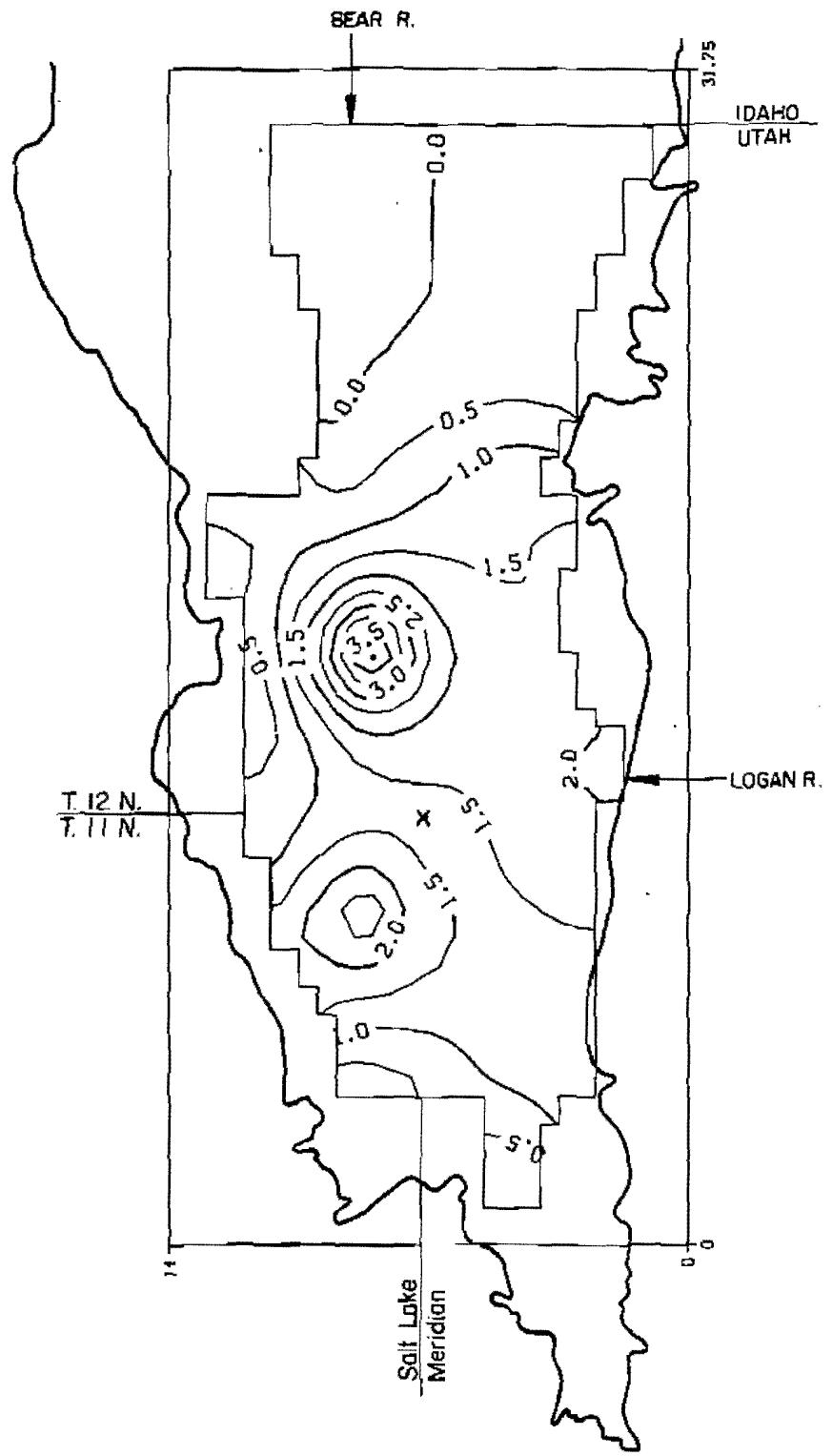


Figure 23. Water level change map of confined aquifer of predictive simulation 2.

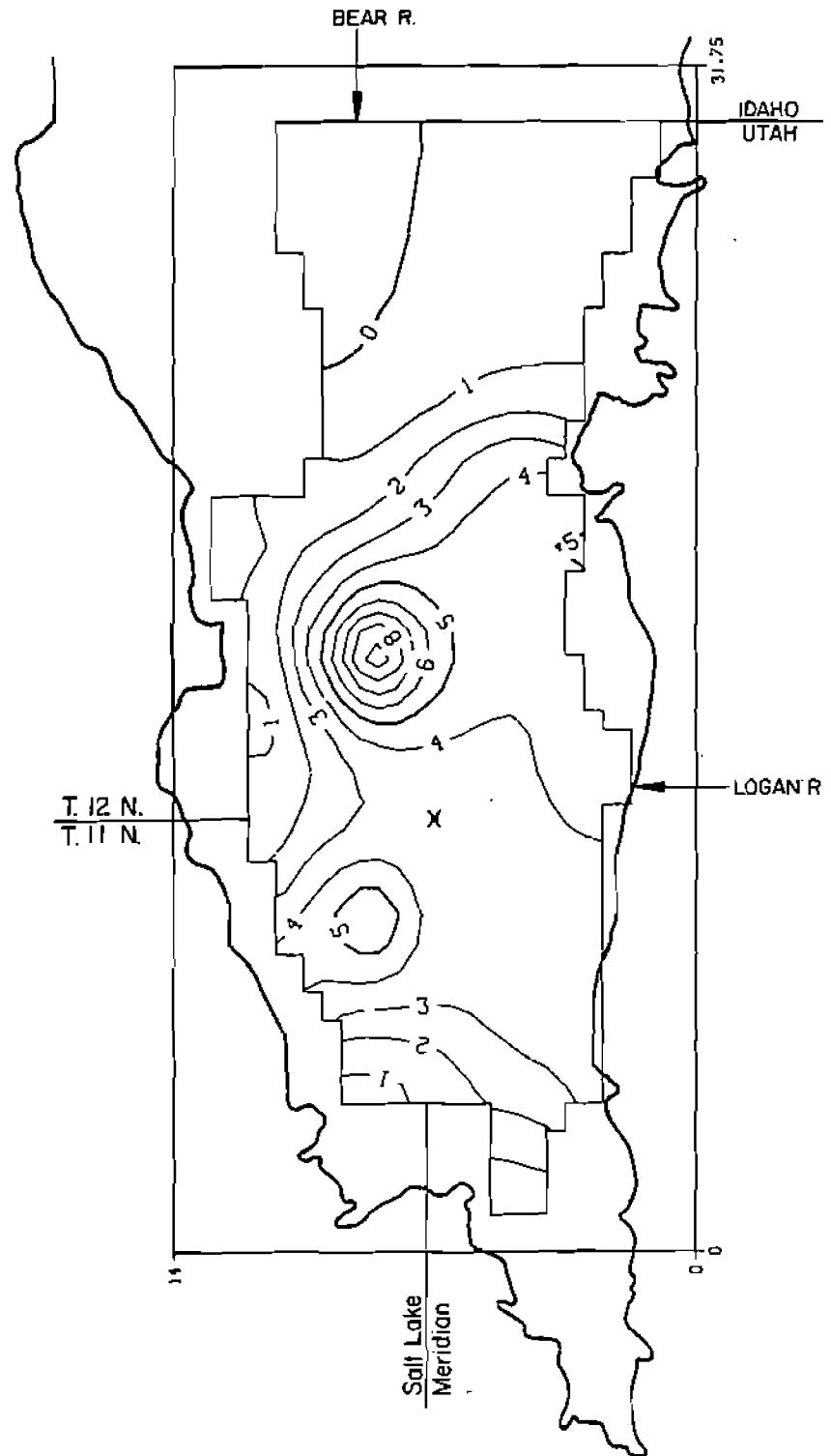


Figure 24. Water level change map of confined aquifer of predictive simulation 3.



SUMMARY, CONCLUSIONS, AND RECOMMENDATIONS

Orderly groundwater development to supply increasing demands requires better information on aquifer systems and tools that can be used to predict how they will respond to different management plans. Quantitative prediction contributes to more efficient water use decisions. This report describes the development, calibration, and use of a quantitative, predictive management model for the groundwater basin of Cache Valley in northern Utah.

More physical information and field data simplify model calibration. In Cache Valley, complicated geologic conditions, a large number of pumping and flowing wells without discharge information on pumping rates and pumping patterns, and limited information on other parameters added to the work required in calibrating the model.

The development of the Cache Valley groundwater model as described above illustrates how a model can be adapted, calibrated, and applied, given complicated hydrologic, hydraulic and geologic conditions in the model area and limited historical data and field measurements. The model represents a complex natural system with one unconfined and one confined aquifer. The single layer confined aquifer represents several real aquifer layers by using equivalent storage properties. The potentiometric head of the hypothetical single confined aquifer represents average head conditions over several separate confined aquifers. River nodes, spring nodes and constant head nodes are used to simulate real interactions of rivers, streams, springs or reservoirs with the aquifers. The river channels of the Bear River and Cub River were represented by constant

head nodes rather than river nodes, as a strategy to separate the two unconfined aquifers and save computer time and storage. Modification of the computer code by adding "evapotranspiration" was necessary because phreatic evapotranspiration plays such an important role in groundwater conditions in Cache Valley. The large number of wells and the sparse information about well discharge and pumping duration required lumping the drafts from many small wells together at nodes so as to keep the total annual draft in line with the known water budget. This method is believed to be accurate enough for the model construction under the limitations of time and financial support for the project.

The mouths of the Logan River and Smithfield Canyon are two of the major groundwater recharge zones. Large amounts of water enter the basin there because of the high transmissivity (over 100,000 ft² per day) and the availability of recharge from the rivers. Smaller amounts of recharge occur at the mouths of smaller, intermittent or ephemeral streams, such as Blacksmith Fork, Providence Canyon, High Creek, Cherry Creek, Summit Creek, Spring Creek, etc.

The quantity of water entering Cache Valley is equal to the quantity leaving the valley plus or minus the change in storage within the valley. Under natural conditions, the change in groundwater storage is small, and discharge is approximately equal to recharge. Deep subsurface outflow from the valley is believed to be negligible. The average annual amount of water withdrawal from wells in Cache Valley is about 26,000 acre-feet, and about 82

percent of the amount is in Utah. This percentage is an average number estimated from the records of the past years. According to the model, the amount of recharge into the confined aquifer less the pumpage is the total amount of upward leakage, since there is no subsurface outflow and an impermeable bottom boundary is assumed in the model. This amount of upward leakage is about 58,000 acre-feet annually and represents a conservative estimate of the additional groundwater that can be withdrawn from the confined aquifer. However, increased withdrawals will lower the head and diminish the discharge from some flowing wells in the artesian areas. Increased withdrawals would also dry up some wet areas and reduce nonproductive evapotranspiration. This pumping could, however, have an adverse effect on wildlife wet lands habitat and would have to be managed carefully. The model provides a quantitative tool for calculating these effects.

The Smithfield-Hyrum-Wellsville area (area 1 in Figure 3) is the largest and most productive aquifer system in Cache Valley. Although this overflowing groundwater area is the most intensely developed in the valley, little long-term change in water levels has occurred (Bjorklund and McGreevy 1971). This area has great potential for further groundwater development because it has high transmissivity (from about 10,000 to over 100,000 ft² per day) and is supplied large amounts of water by recharge from rivers and streams into permeable sands and gravels at the canyon mouths. Other significant recharge is from the irrigation canals which are located along the edges of the valley where water infiltrates most readily. Probably the best sites for developing future large wells are around the mouths of Smithfield Canyon, the Logan River, and Blacksmith Fork. The water level change maps resulting from the predictive simulations shown in Figures 22, 23 and 24 give some indication of water level changes under future development.

The grid system map, shown in Figure 13, summarizes the locations and numbers of existing wells within each grid cell area. This map is helpful in visualizing the effects of well pumpage on drawdown. The observed water level change map from March 1969 to March 1970 in Figure 18a shows three regions with more drawdown than surrounding areas. Comparing this with the well location map of Figure 13, these regions are also observed to have more wells. Apparently, the larger drawdown resulted from larger pumpage because of the many wells in the region. While there is no evident overdraft, in some of these intensive pumping centers, attention should be given to controlling local development so as to reduce the extra drawdown due to well interference.

To improve the model and increase the reliability of its predictions, future groundwater investigations in Cache Valley should include more detailed study of the geologic formations, especially for the Bear River delta area, south of Utah-Idaho state line and between the Bear River and Cub River channel. No really deep wells have been drilled in this area. Potentially productive confined aquifers should lie beneath the delta deposits in the older deposits, but they have not been explored. More effort should go into collecting accurate well logs for all new wells that are drilled.

Another region needing more investigation is near Benson where some deeper wells are located. The more than 1000 feet thick Quaternary deposits need further definition. In the present model, the simulated water levels of the confined aquifer in this area are somewhat lower than the observed water levels in the deepest wells, due to the upward groundwater gradient in the deeper aquifers that is not now simulated.

An expanded observation well network needs to be established where water levels can be measured in each

layer annually or monthly. Well pumping tests also should be done near Benson, Wellsville, Amalga, and in the Bear River delta area near the state line in order to obtain better aquifer characteristics information. More complete data on discharge schedules for both pumping and flowing wells should be collected to improve the estimates of the total draft and its distribution during the year.

After several years of collecting additional data, it could be used

to recalibrate and to improve the present yearly model. It might also be possible, using the new data and simulation information from the improved yearly model, to develop a more useful monthly or seasonally model for the Cache Valley in Utah. The model should also be extended to cover the portion of Cache Valley in Idaho. The techniques and methods developed for the Cache Valley model could also be applied to other geologically and hydrologically similar areas.



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Appendix A
Computer Program Listing


```

13.          IF (IODE.EQ.1) GO TO 60
14.          ISUM=ISUM+ISIZ
15.          IO=IO
16.          JO=JO
17.          KO=KO
18.          GO TO 70
19. 60 ISUM=ISUM+1
20.          IO=1
21.          JO=1
22.          KO=1
23. 70 IF (NSP.EQ.0) GO TO 75
24.          L(24)=ISUM
25.          ISUM=ISUM+ISIZ
26.          L(25)=ISUM
27.          ISUM=ISUM+SP
28.          L(26)=ISUM
29.          ISUM=ISUM+SP
30.          GO TO 76
31. 75 L(26)=ISUM
32.          ISUM=ISUM+1
33.          L(27)=ISUM
34.          ISUM=ISUM+1
35.          L(28)=ISUM
36. 76 IF (NRIV.EQ.0) GO TO 77
37.          L(29)=ISUM
38.          ISUM=ISUM+ISIZ
39.          L(30)=ISUM
40.          ISUM=ISUM+NRIV
41.          L(31)=ISUM
42.          ISUM=ISUM+NRIV
43.          L(32)=ISUM
44.          ISUM=ISUM+NRIV
45.          GO TO 71
46. 77 L(29)=ISUM
47.          L(30)=ISUM+1
48.          L(31)=ISUM+2
49.          L(32)=ISUM+3
50.          ISUM=ISUM+4
51. 71 IF (IALT.EQ.1) GO TO 72
52.          L(33)=ISUM
53.          ISUM=ISUM+1
54.          L(34)=ISUM
55.          ISUM=ISUM+1
56.          L(35)=ISUM
57.          ISUM=ISUM+1
58.          GO TO 79
59. 72 L(33)=ISUM
60.          ISUM=ISUM+ISIZ
61.          L(34)=ISUM
62.          ISUM=ISUM+ISIZ
63.          L(35)=ISUM
64.          ISUM=ISUM+ISIZ
65. 78 IF (IEWP.EQ.1) GO TO 66
66.          L(36)=ISUM
67.          ISUM=ISUM+IK1
68.          GO TO 77
69. 66 L(36)=ISUM
70. 79 WRITE (6,170) ISUM
71.          --PASS INITIAL ADDRESSES OF ARRAYS TO SUBROUTINES--
72.          L14(1)
73.          L24(1)
74.          L34(1)
75.          L44(1)
76.          L54(1)
77.          L64(1)
78.          L74(1)
79.          L84(1)
80.          L94(1)
81.          L104(1)
82.          L114(1)
83.          L124(1)
84.          L134(1)
85.          L144(1)
86.          L154(1)
87.          L164(1)
88.          L174(1)
89.          L184(1)
90.          L194(1)
91.          L204(1)
92.          L214(1)
93.          L224(1)
94.          L234(1)
95.          L244(1)
96.          L254(1)
97.          L264(1)
98.          L274(1)
99.          L284(1)
100.         L294(1)
101.         L304(1)
102.         L314(1)
103.         L324(1)
104.         L334(1)
105.         L344(1)
106.         L354(1)
107.         L364(1)
108.         CALL DATA(Y(L1),Y(L2),Y(L3),(L4),Y(L5),Y(L6),Y(L7),Y(L8),
109.           Y(L9),Y(L10),Y(L11),Y(L12),Y(L13),Y(L14),Y(L15),Y(L16),
110.           Y(L17),Y(L18),Y(L19),Y(L20),Y(L21),Y(L22),Y(L23),Y(L24),
111.           Y(L25),Y(L26),Y(L27),Y(L28),Y(L29),Y(L30),Y(L31),Y(L32))
112.         L184(1)
113.         L204(1)
114.         L224(1)
115.         CALL STEP(Y(L1),Y(L2),Y(L3),Y(L4),Y(L5),Y(L6),Y(L7),Y(L8),
116.           Y(L9),Y(L10),Y(L11),Y(L12),Y(L13),Y(L14),Y(L15),
117.           Y(L16),Y(L17),Y(L18),Y(L19),Y(L20),Y(L21),Y(L22),Y(L23),
118.           Y(L24),Y(L25),Y(L26),Y(L27),Y(L28),Y(L29),Y(L30))
119.         L184(1)
120.         L204(1)
121.         L224(1)
122.         L244(1)
123.         L264(1)
124.         L284(1)
125.         L294(1)
126.         L304(1)
127.         CALL SUBP(Y(L1),Y(L2),Y(L3),Y(L4),Y(L5),Y(L6),Y(L7),Y(L8),
128.           Y(L9),Y(L10),Y(L11),Y(L12),Y(L13),Y(L14),Y(L15),
129.           Y(L16),Y(L17),Y(L18),Y(L19),Y(L20),Y(L21),Y(L22),Y(L23),
130.           Y(L24),Y(L25),Y(L26),Y(L27),Y(L28),Y(L29),Y(L30))
131.         CALL COEF(Y(L1),Y(L2),Y(L3),Y(L4),Y(L5),Y(L6),Y(L7),Y(L8),
132.           Y(L9),Y(L10),Y(L11),Y(L12),Y(L13),Y(L14),Y(L15),
133.           Y(L16),Y(L17),Y(L18),Y(L19),Y(L20),Y(L21),Y(L22),Y(L23),
134.           Y(L24),Y(L25),Y(L26),Y(L27),Y(L28),Y(L29),Y(L30))
135.         CALL TRN(Y(L1),Y(L2),Y(L3),Y(L4),Y(L5),Y(L6),Y(L7),Y(L8),
136.           Y(L9),Y(L10),Y(L11),Y(L12),Y(L13),Y(L14),Y(L15),
137.           Y(L16),Y(L17),Y(L18),Y(L19),Y(L20),Y(L21),Y(L22),Y(L23),
138.           Y(L24),Y(L25),Y(L26),Y(L27),Y(L28),Y(L29),Y(L30))

```

```

      C START COMPUTATIONS
      C---READ AND WRITE DATA FOR GROUPS II AND III---
      C CALL DATINI
      IHN=1
      HJ=1040
      DO 80 K1,K0
      LOCN=(2)+(K-1)*HJ
      80 CALL ARRAY(Y(LDC),INFT(1,2),IOFT(1,1),NAME(1),IRN,DUN)
      DO 90 K=K1,K0
      LOCN=(5)+(K-1)*HJ
      90 CALL ARRAY(Y(LDC),INFT(1,1),IOFT(1,2),NAME(7),IRN,DUN)
      DO 100 K=1,K0
      LOCN=(4)+(K-1)*HJ
      L1=L((19)+K-1)
      L2=L((17)+2*K-1)
      CALL ARRAY(Y(LDC),INFT(1,3),IOFT(1,2),NAME(13),IPN,DUN)
      Y(L1)=DUN(1)
      Y(L2)=DUN(2)
      Y(L3)=DUN(3)
      100 WRITE (6,100) K,Y(L1),Y(L2),Y(L3)
      IF (ITK.NE.10) 100 GO TO 129
      DO 110 K=1,K1
      LOCN=(8)+(K-1)*HJ
      110 CALL ARRAY(Y(LDC),INFT(1,1),IOFT(1,3),NAME(19),IPN,DUN)
      120 IF (IWATER.NE.10) 120 GO TO 130
      K=K0
      CALL ARRAY(Y(LDC),INFT(1,1),IOFT(1,3),NAME(25),IPN,DUN)
      CALL ARRAY(Y(LDC),INFT(1,1),IOFT(1,1),NAME(31),IRN,DUN)
      130 IF (IEVAP.NE.10) 130 GO TO 132
      DO 131 K=1,K0
      LOCN=(25)+(K-1)*HJ
      131 CALL ARRAY(Y(LDC),INFT(1,1),IOFT(1,4),NAME(37),IPN,DUN)
      132 IF (IEVAP.NE.10) 132 GO TO 135
      K=K0
      CALL ARRAY(Y(LDC),INFT(1,2),IOFT(1,1),NAME(43),IPN,DUN)
      135 CALL DOUT
      IF (NSP.NE.0) CALL DOUT(NSP)
      IF (NRIV.NE.0) CALL DOUT(NRIV)

      C---COMPUTE TRANSMISSIBILITY FOR UNCONFINED LAYER---
      C IF (IWATER.EQ.10) CALL TRANS(1)

      C---COMPUTE T COEFFICIENTS---
      C CALL TCOF

      C---COMPUTE ITERATION PARAMETERS---
      C CALL ITCR
      GO TO 142

      C---READ RECHARGE FLUX FOR A NEW PUMPING PERIOD---
      140 DO 141 K=1,K0
      LOCN=(25)+(K-1)*HJ
      141 CALL ARRAY(Y(LDC),INFT(1,1),IOFT(1,4),NAME(37),IPN,DUN)

      C---READ TIME PARAMETERS AND PUMPING DATA FOR NEW PUMPING PERIOD---
      142 CALL NEVPER
      C
      K=0
      IFHAL=0
      C---START NEW TIME STEP COMPUTATIONS---
      150 CALL NEVSTP
      C---START NEW ITERATION IF MAXIMUM NO. ITERATIONS NOT EXCEEDED---
      C CALL NEVITA
      C---PRINT OUTPUT AT DESIGNATED TIME STEPS---
      C CALL OUTPUT
      C---LAST TIME STEP IN PUMPING PERIOD?---
      C IF (IFHAL.NE.1) GO TO 150
      C---CHECK FOR NEW PUMPING PERIOD---
      C IF (NP.LT.NPER) GO TO 140
      C STOP
      C---FORMATS---

      160 FORMAT (6I10)
      170 FORMAT ('0',14X,'WORDS OF VECTOR Y USED ',I7)
      180 FORMAT ('0',14X,'NUMBER OF ROWS ',I5,10X,'NUMBER OF COLUMNS ',I5,
     1 /6IX,'NUMBER OF LAYERS ',I5,10X,'MAXIMUM PERMITTED NUMBER OF ITR
     1 CATIONS ',I5,10X,'NUMBER OF CONSTANT HEAD NODES ',I5,
     1 /5X,'NUMBER OF SPRING NODES ',I5,
     1 /5X,'NUMBER OF RIVER NODES ',I5)
      190 FORMAT ('1',I24,I24)
      200 FORMAT (20E4)
      210 FORMAT (16I4,1X)
      220 FORMAT ('SIMULATION OPTIONS: ',15(A4,2X))
      230 FORMAT (1H0,44X,'DIRECTIONAL TRANSMISSIBILITY MULTIPLICATION FACTORS
     1 FOR LAYER ',I3,17AX,'X =',015.7/78X,'Y =',015.7/78X,'Z =',015.7)
      EHD
      SUBROUTINE DATA(KNI,STR1,OLD,T,S,TR,TC,TM,WELL,DELY,RELY,DELZ,FAC
     1,TFLR,BOTTOM,ORE,LD,FLD,CLD,DR,RH,RC,RR,R,INELL,RCF,QFD)
      READ AND WRITE DATA

      SPECIFICATIONS:
      REAL *4 IDR,MUELL
      CHARACTER *4 INFT,IOFT,IN
      CHARACTER *80LABEL,YLABEL,TITLE,RESUR
      CHARACTER *41CKH,V1,V2,V3,DUDIT,IEON,IEVAP,IMLT,IMDP,INAPP
      CHARACTER *41DRIV,IMAD,IFLO,IMK1,IMK2,IWATER,IPFE,IPUI,IPCD,ITK
      CHARACTER *15TPRINT,BLANK

      COMMON /INTEGER/ IO,JO,KO,II,JL,K1,I-J-K,NPER,KTH,ITMAX,LENGTH,K3,N

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16.      88      LUEL, MUNT, IFINAL, ITKT, IHEAD, IDRAW, IPLO, IPRR, IZ, JZ, KZ, IMAX, ITKX1, IC
17.      89      CH, ITK1, IDK1, IWATER, TONE, IP, JP, IO, JK, IK, JK, KS, IFU1, IFU2, ITK, IHO
18.      90      S, IDEL, IEPMP, IALY, IIMP, IHEP
19.      91      COMMON /SPARAY/ THAZ, CDT, DELT, DRR, ITEST, SUM, SUMP, DR, OET, ETDIST
20.      92      COMMON /SARRA/ ICHK(16), LEVEL(19), LEVEL(2)
21.      93      COMMON /DV/ ETFLXT, STORT, ORET, CHT, CHT, RFLXT, PUMPT, CFLXT, SFXT
22.      94      COMMON /PR/ XLABEL(3), YL48EL(6), TITLE(8), IOH, MESUR, PRNT(122), BLANK
23.      95      1481, DIGIT(122), IF1481, VF2(5), VF3(7), VSCALE, DINCH, SYAL(7), RH(10),
24.      96      CYN(13), WA(42) WH(2), CTSCALE, FACT1, FACT2
25.      97
26.      98      DIMENSION PHI(IO,JO,KO), STRT(IO,JO,KO), OLD(IO,JO,KO), TK(IJ,JO,KO
27.      99      1), B(IJ,JO,KO), TR(IJ,JO,KO), TC(IJ,JO,KO), TK(JK,JK,KS), WELL(IJ,
28.      100     2,JO,KO), DELK(JO), DELY(IJ), DEL(JK,JO), FACTK(0-3), FDR(IP,JP), BOT
29.      101     3,THK(IP,JP), ORE(IJ,JO,KD), TF(3), AL(10,JO), ZH(6), IOFT(9), INFT(2)
30.      102     4, ID(IJ,JO,KO), FLD(1), ELD(1), IDC(IJ,JO,KO), RH(1), PC(1), RS(1)
31.      103     5, SWELL(IJ,JO,KO), HUELL(IJ,JO,KO), RCF(IJ,JO,KO), ORND(IJ,JO)
32.      104
33.      105      RETURN
34.      106      *****-----*****-----*****-----*****-----*****-----*****
35.      107      ENTRY DATAIN
36.      108      *****-----*****-----*****-----*****-----*****-----*****
37.      109      ----READ AND WRITE SCALAR PARAMETERS-----
38.      110      READ (5,330) IPER, KTH, ERR, LENGTH, OET, ETDIST
39.      111      WRITE (6,340) IPER, KTH, ERR, OET, ETDIST
40.      112      READ (5,440) NSCALE, VSCALE, DINCH, FACT1, (LEVEL(I), I=1,2), FACT2, /LE
41.      113      LEVEL(1), I=1,9), MESUR
42.      114      IF (NSCALE.NE.0.) WRITE (6,470) NSCALE, VSCALE, MESUR, DINCH, FA
43.      115      ICT1, LEVEL1, FACT2, LEVEL2
44.      116
45.      117      ----READ CUMULATIVE MASS BALANCE PARAMETERS-----
46.      118      READ (5,450) SUM, SUMP, PUMPT, CFLXT, ORET, CHT, CHT, RFLXT, STORT, ETFL
47.      119      1XT, SFXT
48.      120      IF (IDK1.EQ.1CHK(4)) GO TO 20
49.      121      IF (IFU1.NE.1CHK(6)) GO TO 20
50.      122
51.      123      ----READ INITIAL HEAD VALUES FROM CARDS-----
52.      124      DO 10 K=1,KO
53.      125      DO 10 I=1,IO
54.      126      10 READ (5,360) (PHI(I,J,K), J=1,JO)
55.      127      GO TO 30
56.      128
57.      129      ----READ INITIAL HEAD AND MASS BALANCE PARAMETERS FROM DISK-----
58.      130      READ (14) PHI, SUM, SUMP, PUMPT, CFLXT, ORET, CHT, CHT, RFLXT, STORT, ETFL
59.      131      1XT, SFXT
60.      132      READ (5,360) SUM
61.      133      DO 40 K=1,KO
62.      134      WRITE (6,440) K
63.      135      DO 40 I=1,IO
64.      136      40 WRITE (6,360) I, (PHI(I,J,K), J=1,JO)
65.      137
66.      138      DO 40 K=1,KO
67.      139      DO 40 I=1,IO
68.      140      DO 40 J=1,JO
69.      141      WELL(I,J,K)=0.
70.      142      TR(I,J,K)=0.
71.      143      TC(I,J,K)=0.
72.      144      IF (X(K,E,K)) TK(I,J,K)=0.
73.      145      CONTINUE
74.      146      RETURN
75.      147      *****-----*****-----*****-----*****-----*****-----*****
76.      148      ENTRY ARRAY(A,INFT,IOFT,IM,IRN,TF)
77.      149      *****-----*****-----*****-----*****-----*****-----*****
78.      150      READ (5,340) FAC, IVAR, IPRN, IF, IRECS, IRECD
79.      151      IC=4*IRECS+2*IVAR+IPRN+1
80.      152      DO 100 I=1,IO
81.      153      DO 80 J=1,JO
82.      154      A(I,J)=FAC
83.      155      WRITE (6,290) IN, FAC, K
84.      156      DO 140
85.      157      90 IF (IC.EQ.3) WRITE (6,290) IM, K
86.      158      DO 110 I=1,IO
87.      159      READ (5,INFT) (A(I,J), J=1,JO)
88.      160      DO 100 J=1,JO
89.      161      100 A(I,J)=A(I,J)*FAC
90.      162      110 IF ((IC.EQ.3) WRITE (6,IOFT) I,(A(I,J),J=1,JO)
91.      163      DO 140
92.      164      120 READ (5,IRN) A
93.      165      IF (IC.EQ.6) GO TO 140
94.      166      WRITE (6,290) IRN, K
95.      167      DO 130 I=1,IO
96.      168      130 WRITE (6,IOFT) I,(A(I,J), J=1,JO)
97.      169      IF (IRECD.EQ.1) WRITE (5,IRN) A
98.      170      IRN=IRN+1
99.      171      RETURN
100.      172      *****-----*****-----*****-----*****-----*****-----*****
101.      173      ENTRY HDT
102.      174      *****-----*****-----*****-----*****-----*****-----*****
103.      175      DO 150 K=1,KO
104.      176      DO 150 I=1,IO
105.      177      DO 150 J=1,JO
106.      178      IF ((I.EQ.1).OR.(J.EQ.1).OR.(J.EQ.I)) TK(I,J,K)=0.
107.      179      IF ((IXK1.NE.IDK1).AND.(IFU1.NE.1CHK(6))) FNL(I,J,K)=STRT(I,J,K)
108.      180      IF ((K.NE.KD).AND.(IWATER.NE.1CHK(6))) GO TO 150
109.      181      IF ((I.EQ.1).OR.(J.EQ.1).OR.(J.EQ.I)) PERMIT(I,J)=0.
110.      182      150 CONTINUE
111.      183      *****-----.....DLX .....
112.      184      READ (5,330) FAC, IVAR, IPRN
113.      185      IF (IVAR.EQ.1) READ (5,330) (DELY(J), J=1,JO)
114.      186      DO 170 J=1,JO
115.      187      IF (IVAR.NE.1) GO TO 160
116.      188      DELX(J)=DELY(J)*FAC
117.      189      DO 170 J=1,JO
118.      190      160 DELX(J)=FAC
119.      191      IF (IPRN.EQ.1.AND.IPRN.NE.1) WRITE (6,370) (DELY(J), J=1,JO)
120.      192      IF (IPRN.EQ.0) WRITE (6,399) FAC
121.      193      *****-----.....DELY .....
122.      194      READ (5,330) FAC, IVAR, IPRN

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128.      IF (IVAR,ED,1) READ (5,IO0) (DELY(I),I=1,IO)
129.      DO 190 I=1,IO
130.      IF (IVAR,NE,1) GO TO 180
131.      DELY(I)=DELY(I)*FAC
132.      GO TO 190
133.      180 DELY(I)=FAC
134.      190 CONTINUE
135.      IF (IVAR,ED,1,AND,IPRN,NE,1) WRITE (6,380) (DELY(I),I=1,IO)
136.      IF (IVAR,ED,0) WRITE (6,310) FAC
137.      C ..... DELZ .....
138.      READ (5,JO0) FAC,IVAR,IPRN
139.      IF (IVAR,ED,1) READ (5,JO0) (DELZ(K),K=1,K0)
140.      DO 210 K=1,K0
141.      IF (IVAR,NE,1) GO TO 200
142.      DELZ(K)=DELZ(K)*FAC
143.      GO TO 210
144.      200 DELZ(K)=FAC
145.      210 CONTINUE
146.      IF (IVAR,ED,1,AND,IPRN,NE,1) WRITE (6,390) (DELZ(K),K=1,K0)
147.      IF (IVAR,ED,0) WRITE (6,320) FAC
148.      C ---INITIALIZE VARIABLES---
149.      J0=0
150.      D0=0
151.      F=0.
152.      H=0.
153.      SUM=0.
154.      I=0.
155.      IF (XSCALE,NE,0.) CALL MAP
156.      RETURN
157.      C ..... DDAT(HSP) .....
158.      ENTRY DDAT(HSP)
159.      C ..... DDAT(HSP) .....
160.      NK=1
161.      DO501 K=1,K0
162.      DO 200 I=1,IO
163.      READ 210, (IDR(I,J,K),J=1,JO)
164.      210 FORMAT (8X,O1.0)
165.      DO 200 J=1,JO
166.      IF (IDR(I,J,K),ED,0.) GO TO 200
167.      IDR(I,J,K) = FLOAT(NK)
168.      NK=NK+1
169.      200 CONTINUE
170.      201 CONTINUE
171.      NK=NK-1
172.      IF (NK,ED,NSP) GO TO 510
173.      PRINT 510,NK,NSP
174.      510 FORMAT (' ERROR IN DDAT(NK,NE,NSP) NK=''',IS,''',NSP=''',IS)
175.      STOP
176.      520 READ J30,FAC
177.      PRINT 521,FAC
178.      521 FORMAT (1X,O1.4)
179.      READ 530, (FLD(I),I=1,NSP)
180.      530 FORMAT (1X,O1.0)
181.      PRINT 540,(FLD(I),I=1,NSP)
182.      540 FORMAT (1X,O1.0)
183.      DO 550 I=1,NSP
184.      FLD(I)=FLDI*FLD(I)*FAC
185.      550 CONTINUE
186.      READ J30,FAC
187.      PRINT 551,FAC
188.      READ 560,(ELD(I),I=1,NSP)
189.      PRINT 560,(ELD(I),I=1,NSP)
190.      560 FORMAT (1X,O1.0)
191.      DO 570 I=1,NSP
192.      ELD(I)=ELDI*ELD(I)*FAC
193.      570 CONTINUE
194.      RETURN
195.      C ..... DDAT(RHIV) .....
196.      ENTRY DDAT(RHIV)
197.      C ..... DDAT(RHIV) .....
198.      NK=1
199.      DO 580 K=1,K0
200.      DO 590 I=1,IO
201.      READ 510,(IDR(I,J,K),J=1,JO)
202.      DO 590 J=1,JO
203.      IF (IDR(I,J,K),ED,0.) GO TO 580
204.      IDR(I,J,K) = FLOAT(NK)
205.      NK = NK + 1
206.      580 CONTINUE
207.      591 CONTINUE
208.      NK = NK - 1
209.      IF (NK,ED,RHIV) GO TO 600
210.      PRINT 585,NK,RHIV
211.      585 FORMAT (' ERROR IN DDAT(NK,NE,RHIV) NK=''',IS,''',RHIV=''',IS)
212.      STOP
213.      600 READ J30,FAC
214.      PRINT 521,FAC
215.      READ 540,(RH(I),I=1,NRIV)
216.      PRINT 540,(RH(I),I=1,NRIV)
217.      DO 610 I=1,NRIV
218.      RH(I) = RH(I)*FAC
219.      READ J30,FAC
220.      PRINT 521,FAC
221.      READ 560,(RB(I),I=1,NRIV)
222.      PRINT 560,(RB(I),I=1,NRIV)
223.      DO 630 I=1,NRIV
224.      RB(I) = RB(I)*FAC
225.      READ J30,FAC
226.      PRINT 521,FAC
227.      READ 575,(RC(I),I=1,NRIV)
228.      575 FORMAT (1OFE.0)
229.      PRINT 575,(RC(I),I=1,NRIV)
230.      575 FORMAT ('1A1X,8.0')
231.      DO 650 I=1,NRIV
232.      RC(I)=RC(I)*FAC
233.      650 RC(I)=RC(I)*FAC
234.      RETURN
235.      C ..... READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIOD-
236.      C ..... READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIOD-
237.      C ..... READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIOD-
238.      C ..... READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIOD-
239.      C ..... READ TIME PARAMETERS AND PUMPING DATA FOR A NEW PUMPING PERIOD-

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140.      READ (5,330) KP, KPM1, HUEL, THAX, NUMT, COLT, DELT
141.      -----COMPUTE ACTUAL DELT AND NUMT-----
142.      DT=DELT/24.
143.      TH=DEL/24.
144.      DO 220 I=1,NUMT
145.      DT=COLT*DT
146.      TH=TH*DT
147.      IF (TH.GE.THAX) GO TO 230
148. 210  CONTINUE
149.  GO TO 240
150. 220  DELT=THAX/TH*DELT
151.  NUMT=I
152. 240  WRITE (6,400) KP, THAX, HULT, DELT, COLT
153.      DELT=DEL/2400.
154.      THAX=THAX*2400.
155.      SUMP=0.0
156.      -----READ AND WRITE WELL PUMPING RATES-----
157.      WRITE (6,410) HUEL
158.      IF (HUEL.EQ.0) GO TO 260
159.      IF (KALT.EQ.1)CK(13) GO TO 261
160.      DO 220 II=1,HUEL
161.      READ (5,330) K,I,J,WELL(I,J,K)
162.      WRITE (6,420) K,I,J,WELL(I,J,K)
163.      220  WELL(I,J,K)=WELL(I,J,K)/(DELX(I)*DELY(I))
164.      GO TO 260
165.      -----ALTERNATIVE-----
166. 241  DO 251 K=1,KD
167.      DO 251 I=1,10
168.      DO 251 J=1,10
169.      RCF(I,J,K)=0.
170.      HUELL(I,J,K)=0.
171. 251  WELL(I,J,K)=0.
172.      READ(5,330) AMELL,IMUEL,BUC,FBUELL,NCF,FCF
173.      IF (AMELL.EQ.9999.9) GO TO 240
174.      DO 240 II=1,IMUEL
175.      READ(5,330) K,I,J,WELL(I,J,K)
176.      DUELL(I,J,K)=WELL(I,J,K)*BUC
177. 242  WRITE(6,420) K,I,J,DUELL(I,J,K)
178.      DO 247 II=1,NCF
179.      READ(5,330) K,I,J,RCF(I,J,K)
180.      RCF(I,J,K)=RCF(I,J,K)*FCF
181. 247  WRITE(6,420) K,I,J,RCF(I,J,K)
182.      WRITE(6,250) AMELL,IMUEL,BUC,FBUELL,NCF,FCF
183. 250  FORMAT(1HO/6D10.4)
184. 251  DO 263 K=1,KD
185. 252  DO 263 I=1,10
186. 253  READ(5,440) (HUELL(I,J,K),J=1,10)
187. 254  FORMAT(30D4.0)
188. 255  DO 254 K=1,KD
189. 256  DO 254 I=1,10
190. 257  DO 254 J=1,10
191. 258  264  WELL(I,J,K)=WELL(I,J,K)+AMUEL*DUELL(I,J,K)*RCF(I,J,K)/
192. 1*(DELX(I)*DELY(I))
193. 265  GO TO 245
194. 266  RETURN
195.      -----FORMATS-----
196.
197. 267  FORMAT (1HO.5X,6A4,/,*,015.7,' FOR LAYER',I3)
198. 268  FORMAT (1H1.4X,6A4,/, MATRIX, LAYER',I3/4X,41(''--'))
199. 269  FORMAT ('0.72X*DELT',*,015.7)
200. 270  FORMAT ('0.72X*DELY',*,015.7)
201. 271  FORMAT ('0.72X*DELZ',*,015.7)
202. 272  FORMAT (1D10.0)
203. 273  FORMAT ('0.72X/NUMBER OF PUMPING PERIODS',*,1D7.4X, 'TIME STEPS',I3)
204. 274  IETECH PRNTOUTS *,I3/5X, 'ERROR CRITERIA FOR CLOSURE',*,G15.7
205. 275  254X, 'EVAPOTRANSPIRATION RATE',*,015.75X, 'EFFECTIVE DEPTH OF ET',*
206. 276  3'*,015.7)
207. 277  259 FORMAT ('0.72X*2076.1/(3X,2076.1)')
208. 278  360 FORMAT (1D10.4)
209. 279  370 FORMAT (1H1.4X,40HGRID SPACING IN PROTOTYPE IN X DIRECTION/4X,40
210. 280  11-'//1D10.0)
211. 281  380 FORMAT (1H1.4X,40HGRID SPACING IN PROTOTYPE IN Y DIRECTION/4X,40
212. 282  11-'//1D10.0)
213. 283  390 FORMAT (1H1.4X,40HGRID SPACING IN PROTOTYPE IN Z DIRECTION/4X,40
214. 284  11-'//1D10.0))
215. 285  400 FORMAT ('1.0/5X, 'PUMPING PERIOD NO.',I4, '1 DAYS',5X,2D1
216. 286  1-//232, 'NUMBER OF TIME STEPS',*,1D7.5X, 'DELT IN HOURS',*,F10.3/
217. 287  233X, 'MULTIPLIED FOR DELT',*,1D7.3)
218. 288  410 FORMAT ('1.0/5X, 'WELLS',*,5X,PF11-1/50X, 'K',PF11-1X,9,*,J
219. 289  1PF10.4)
220. 290  420 FORMAT (4IX,310,2E13.5)
221. 291  430 FORMAT ('1.0/40X, 'CONTINUATION - HEAD AFTER ',4D9.7, SEC PUMPED
222. 292  1F10.4,5X,2E13.5)
223. 293  440 FORMAT ('1.0/5X, 'INITIAL HEAD MATRIX, LATER',I3/5X,30(''--'))
224. 294  450 FORMAT (4D20.10)
225. 295  460 FORMAT (3G10.9-3(010.0?21,1X,0)AB)
226. 296  470 FORMAT ('0.72X*1N ALPHAMERIC MAP',*,40X, 'MULTIPLICATION FACTOR FO
227. 297  IR X DIMENSION',*,015.7/40X, 'MULTIPLICATION FACTOR FOR Y DIMENSION
228. 298  2',*,015.7/50X, 'MAP SCALE IN UNITS OF ',*1H1.5X, 'NUMBER OF ',*,08, 'P
229. 299  254X, 'MULTIPLICATION FACTOR FOR MM/INCH',*,015.7/43X, 'MULTIPLICATION FACTOR FOR MM/MM',*,015.7-
230. 300  4' PRINTED FOR LAYERS',*,012.47X, 'MULTIPLICATION FACTOR FOR HEAD',*,0
231. 301  515.7, 'PRINTED FOR LAYERS',*,012.47X
232. 302  END
233.
234. 303  SUBROUTINE STEP(PHI,STRT,OLD,T,S,IP,TC,TX,WELL,DELX,DELY,DELC,FACT
235. 304  1,DM,TEST3)
236.
237. 305  INITIALIZE DATA FOR A NEW TIME STEP AND PRINT RESULTS
238.
239. 306  SPECIFICATIONS:
240. 307  CHARACTER *8XLABEL, *1LABEL, *TITLE, *HESUP
241. 308  CHARACTER *4ICHK, *F1.4E1, *F2.4E3, *MOIT, *TECH, *EVAP, *TINF, *ISAPP
242. 309  CHARACTER *4ITRAU, *HEAD, *FLD, *INC, *1OK2, *1WATER, *IORE, *IPU, *IPC, *ITV
243. 310  CHARACTER *1STH, *PRNT, *BLANK

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124.      IF (IDR2.EQ.1)K(3), WRITE (4,PN1,SUM,SUMP,PUMPT,CPFLXT,ORET,CNST
125.      ,CNOT,PFLLXT,STORT,ETFLXT,BFLXT
126.
127.      C   -----PUNCHED OUTPUT-----
128.      IF (IPU2.NE.ICK(4)) GO TO 160
129.      IF (IERR.EQ.2) GO TO 140
130.      WRITE (7,20) SUM,SUMP,PUMPT,CPFLXT,ORET,CNST,CNOT,PFLLXT,STORT,ETFLXT
131.      KLYT,SPFLXT
132.      DO 150 I=1,10
133.      DO 150 J=1,10
134.      150 WRITE (7,20) (PHI(I,J,K),J=1,10)
135.      160 IF (IERR.EQ.2) STOP
136.      RETURN
137.
138.
139.
140.
141.
142.
143.
144.      C   -----FORMAT-----
145.      170 FORMAT ('0',I4,15(1X,F7.2)/(5X,15(1X,F7.2)))
146.      180 FORMAT ('MAXIMUM HEAD CHANGE FOR EACH ITERATION'//',39(''-')//'
147.      181.           1,10F12.4)
148.      190 FORMAT ('1',5X,'HEAD MATRIX LAYER',I3,5DX,2I(''-'))
149.      200 FORMAT ('1',5X,'DRAWDOWN LAYER',I3,5DX,1B(''-'))
150.      210 FORMAT (1H1,4X,57(''-')/15X,'1',14X,'TIME STEP NUMBER =',19,14X,
151.      211.           1'/15X,57(''-')/50X,'2,3,4 OF TIME STEP IN SECONDS =',F14.2,5DX,1D
152.      212.           STAL SIMULATION TIME IN SECONDS =',F14.2,5DX,1D,MINUTES =',F14.2,5DX,1H
153.      213.           HOURS =',F14.2,5DX,5DAYS =',F14.2,5DX,'YEARS =',F14.2,5DX,1D, 'YEARS =',F14.2,5DX
154.      214.           'OF CURRENT PUMPING PERIOD IN DAYS =',F14.2,5DX,1D
155.      220 FORMAT (10F8.1)
156.      230 FORMAT (4G20.10)
157.      240 FORMAT ('0'LINE STEP 1',40I3)
158.      250 FORMAT ('0'ITERATIONS',40I3)
159.      260 FORMAT ('1',10(''-'))
160.
161.      C   -----
162.      SUBROUTINE SOLVE(PHI,STRT,OLD,T,B,TR,IC,TK,WELL,DELY,DELT,DELZ,FAC
163.      17.EL,FL,OL,N,XI,TEST3,ORE,PERM,ID,FLD,ELD,ICR,RH,PC,RB,ISPRD,
164.      21RIU,BOTTOM,GRND)
165.
166.      C   -----
167.      C   SOLUTION BY THE STRONGLY IMPLICIT PROCEDURE
168.
169.
170.      C   -----
171.      C   SPECIFICATIONS:
172.      REAL *4 ID, IDR
173.      REAL PHI, RHQ, B, G, F, H, Z, SU, PHOP, U, UNIN, RHO1, RHO3, XPNRT, YPART
174.      REAL UX, UUY, ZPART, UMAX, X1, Y1, Z1, TCR, TCR1, TCR2
175.      REAL EAL, N, OL, A, C, D, UU, TUAD, DL, RES, SURF, GLX1, SPHI
176.      CHARACTER *41CKN, TCRNT, TCRNP, IACT, TCRPP, IAPPP
177.      CHARACTER *41CKN, HEAD, IFLD, IDK1, IDK2, IAWTR, ICRE, IPUI, IPU2, ITK
178.
179.      COMMON /INTEGR/ IO,JO,KD,E1,J1,K1,I,J,K,NPER,KT,ITMAX,LENGTH,ZP,N
180.      IUEL,NINT,ITHAL,IT,KT,THEAD,IPAW,IFLO,IERR,IC,J2,IC,ITMAX,ITMX1,NC
181.      CH,IK1,IK2,IWATER,IGRS,JP,JP1,JO,JO2,IK,J2,K,IPU1,IPUD,ITK+1D0
182.      J1,EGN,IEAP,IAUT,IMDP,IAHP
183.      COMMON /SPRM/ THMAX, COLT, DELT, ERS, TEST, SUM, SUMP, OR, ORET, ETOIST
184.      COMMON /SARRAY/ ICKH(14),LEVEL(9),LEVEL2(9)
185.
186.      C   -----
187.      DIMENSION PHI(1), STRT(1), OLD(1), T(1), S(1), TR(1), FC(1), TK(1)
188.      1. WELL(1), DELX(1), DELY(1), DELZ(1), FACT(K0/3), PHOP(20), TEST3(
189.      21), BOTTOM(1), EL(1), FL(1), GL(1), Y(1), X(1), ORE(1)
190.      31, EID(1), FLD(1), ELP(1), PERM(1), IDR(1), RH(1), RC(1), RD(1)
191.      41, GRND(1)
192.      RETURN
193.
194.      C   -----
195.      C   ENTRANCE CONDITIONS
196.      ENTRY ITCR
197.      C   -----
198.      C   COMPUTE AND PRINT ITERATION PARAMETERS
199.      200. WRITE (6,240)
200.      240 UNTIL=.EO
201.      241 DELT=.1
202.      242 DELT=.1
203.      243 LENGTH=1
204.      244 MT=10130000
205.      245 NJ=10410
206.      246 XT=3.14159265358979323846264338327950288419716939937510582860
207.      247 YT=3.14157314375000000000000000000000000000000000000000000000000000
208.      248 ZT=3.14159265358979323846264338327950288419716939937510582860
209.      249 RHO1=0.60
210.      250 RHO2=0.60
211.      251 RHO3=0.60
212.      252 DO 40 K=1,K0
213.      253 DO 40 J=2,J1
214.      254 DO 40 I=2,I1
215.      255 N=1+(J-1)*IO+(K-1)*NEJ
216.      256 IF (T(N),EO,0.) GO TO 40
217.      257 D=TR(N)-IN*(DELY(J))
218.      258 P=TR(N)/DELY(J)
219.      259 T=TC(1)-1/(DELY(J))
220.      260 H=TC(2)-1/(DELY(J))
221.      261 SUM=0.E0
222.      262 Z=0.E0
223.      263 IF (K,NE,1) Z=TK(H-1)/DELY(K)
224.      264 IF (X,NE,K0) SU=TK(H0)/DELY(K)
225.      265 TCKMAX=MAX(D,P)
226.      266 TYMAX=MAX(SU,Z)
227.      267 TZMAX=MAX(SU,Z)
228.      268 DEN=MAX(H1,D)
229.      269 IF (DEN.EQ.0.E0) DEN=TCM
230.      270 IF (DEN.EQ.0.E0) GO TO 20
231.      271 RHO1=AMAX1(RHO1,TCK/DEN)
232.      272 RHO2=AMAX1(RHO2,TYMAX/DEN)
233.      273 RHO3=AMAX1(RHO3,TZMAX/DEN)
234.      274 DEPMIN=MIN(D,P)
235.      275 IF (DEN.EQ.0.E0) DEN=TYH
236.      276 IF (DEN.EQ.0.E0) GO TO 30
237.      277 RHO2=AMAX1(RHO2,TYMAX/DEN)
238.      278 DEPMIN=MIN1(SU,Z)
239.      279 IF (DEN.EQ.0.E0) DEN=TCM
240.      280 IF (DEN.EQ.0.E0) GO TO 10
241.      281 RHO3=AMAX1(RHO3,TZMAX/DEN)
242.      282 CONTINUE
243.      283 XPART=XT/(1.E0+RHO1)
244.      284 YPART=YT/(1.E0+RHO2)
245.      285 ZPART=ZT/(1.E0+RHO3)
246.      286 UNIN=AMIN1(UNIN,XPART,YPART,ZPART)

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191.      17 IF (PHI(N).GE.BOTTOM(NKA)) GO TO 131
192.      17 IF (PHI(NKA).GE.BOTTOM(NKA)) OR=OR-SUBOTTOM(NKA)
193.      17 IF (PHI(NKA).GE.BOTTOM(NKA)) GO TO 137
194.      17 EME+SU
195.      17 SU=0
196.      17 GO TO 131
197.      17 EME+SU
198.      17 131 IF (K.NE.KO) GO TO 173
199.      17 IF (PHI(NKB).GE.BOTTOM(NKA)) GO TO 133
200.      17 IF (PHI(N).GE.BOTTOM(NKA)) OR=OR-2*BOTTOM(NKA)
201.      17 IF (PHI(N).GE.BOTTOM(NKA)) GO TO 132
202.      17 EME+Z
203.      17 Z=0
204.      17 133 CONTINUE
205.      17 DL=B(1,1)*UX(EL(NKB))+GL(HEP))
206.      17 CL=D(1,1)*US(FL(NKB))+GL(HJB))
207.      17 C=BL*EL(NKB)
208.      17 G=CL*FL(NKB)
209.      17 UU=CL*US(NKB)
210.      17 U=BL*US(NKB)
211.      17 IF (K.EQ.1) GO TO 140
212.      17 AL=C(1,1)*UX(EL(NKB))+FL(HCB))
213.      17 AVAL=AL*(NKB)
214.      17 TUMAL=FL(HCB)
215.      17 DL=E*(A-C)+U(U+TU-U)-CL*EL(NKB)-BL*FL(NTB)-AL*VOL(HCB)
216.      17 EL(N)=F-U(A+C))/DL
217.      17 FL(N)=(U-U(U+TU))/DL
218.      17 GL(N)=(SU-U(U+U))/DL
219.      17 SUPH=0.0
220.      17 IF (K.NE.KO) SUPH=SUPPHI(NKA)
221.      17 RESA=RESPHI(NKB)-DAPHT(NKB)-EPHI(N)-FISHI(NJB)-HSPHI(HIA)-SUPH-ZIP
222.      17 IHI(N)=RES-AL*V(NKB)-BL*V(NTB)-CL*V(NJB))/DL
223.      17 GO TO 150
224.      17 140 DL=E*(A-C)+U(U+U)-CL*EL(NKB)-BL*FL(HCB)
225.      17 EL(N)=(F-U(C))/DL
226.      17 FL(N)=(U-U(C))/DL
227.      17 GL(N)=(SU-U(U+U))/DL
228.      17 SUPH=0.0
229.      17 IF (K.NE.KO) SUPH=SUPPHI(NKA)
230.      17 RESA=RESPHI(NKB)-DAPHT(NJB)-EPHI(N)-FISHI(NJA)-HSPHI(HIA)-SUPH-HEL
231.      17 IHN=(RES-AL*V(NKB)-BL*V(NTB)-CL*V(NJB))/DL
232.      17 GO TO 150
233.      17 150 CONTINUE
234.      17
235.      17 C — BACK SUBSTITUTE FOR VECTOR XI —
236.      17 DO 160 K=1,KO
237.      17 K=KO-K+1
238.      17 DO 160 I=1,I2
239.      17 I=I-10-I
240.      17 DO 160 J=1,J2
241.      17 J=J-10-J
242.      17 N=I+J-10+(K-1)*NIIJ
243.      17 IF (T(N).EQ.0..OR.S(N).LT.0.) GO TO 160
244.      17 GLXI=0.0
245.      17 IF (K.NE.KO) GLXI=GL(N)*XI(N+H+NIJ)
246.      17 XI(N)=V(N)-EL(N)*XI(N+I)-FL(N)*XI(N+J)-GLXI
247.      17
248.      17 C COMPARE MAGNITUDE OF CHANGE WITH CLOSURE CRITERIA —
249.      17 TDM=ABS(XI(N))
250.      17 IF (TDM.LE.BIG) GO TO 153
251.      17 BIG = TDM
252.      17 IIN = I
253.      17 JIN = J
254.      17 KON = K
255.      17 BIGS = XI(N)
256.      17 153 PHI(N)=PHI(N)+XI(N)
257.      17 160 CONTINUE
258.      17 PRINT 251,IIN,JIN,KON,BIGS
259.      17 IF (BIG.GT.ERR) TEST=1,
260.      17 TEST3(IIT+1)=BIG
261.      17 IF (TEST.EQ.0.) RETURN
262.      17 GO TO 60
263.      17
264.      17 170 DO 200 KK=1,K0
265.      17 K=KK-K+1
266.      17 DO 200 II=1,I2
267.      17 I=I-10-I
268.      17 DO 200 JJ=1,JL
269.      17 HMM=(+I-J-1)*I10
270.      17 HMM=HMM+(K-1)*NNIJ
271.      17 HIA=HMM+1
272.      17 HIB=HMM+1
273.      17 HIC=HMM+10
274.      17 HJD=HMM+10
275.      17 HKD=HMM+10
276.      17 HKD=HKD+10
277.      17 HKD=HKD+10
278.      17 HKD=HKD+10
279.      17 HKD=HKD+10
280.      17
281.      17 C — SKIP COMPUTATIONS IF NODE OUTSIDE AQUIFER —
282.      17 IF (TK(N).EQ.0..OR.S(N).LT.0.) GO TO 299
283.      17
284.      17 C — COMPUTE COEFFICIENTS —
285.      17 D=TR(NKB)/DELY(J)
286.      17 F=TR(N),/DELY(J)
287.      17 B=TC(NKB)/DELY(I)
288.      17 H=TC(N)/DELY(I)
289.      17 SU=0.0
290.      17 Z=0.0
291.      17 IF (K.EQ.1) GO TO 171
292.      17 Z=TK(NKB)
293.      17 IF (IEQN.EQ.1)CHK(I11)=Z*I/DELY(K)
294.      17 IF (K.EQ.KO) GO TO 172
295.      17 SU=TK(N)
296.      17 IF (IEQN.EQ.1)CHK(I11)=SU-SU*TEL(Z)
297.      17 RHO=S(N)/DELT
298.      17 OR=0.
299.      17 UX = 0.
300.      17 ETOB=0.
301.      17 ETOD=0.
302.      17 IF (IRE.EQ.1)CHK(I11)=OR*ORE/I11

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303. 17      IF (K.NE.K0.OR.IEVNP.NE.1)MK(12) GO TO 173
304. 17      C
305. 17      C      COMPUTE EXPLICIT AND IMPLICIT PARTS OF ET RATE—
306. 17      IF (PHI(N).LE.GRD(NMA)-ETDIST) GO TO 173
307. 17      IF (PHI(N).GT.GRD(NMA)) GO TO 177
308. 17      ETUB=ET/ET0ST
309. 17      ET0D=ET0ST/ETDIST-GRD(NMA)
310. 17      GO TO 173
311. 17      C
312. 17      C      ET0D=ET
313. 17      C      173 IF (IRIV.LE.0) GO TO 173
314. 17      C      ND = IFIX(IDR(N))
315. 17      C      IF (ND.EQ.0) GO TO 175
316. 17      C      IF (PHI(N).LT.RB(HD).AND.FHI(N).LT.RH(HD)) GO TO 174
317. 17      C      IF (PHI(N).GT.RH(HD)) GO TO 176
318. 17      C      DR = DR - REND1*RH(HD)-RB(HD)
319. 17      C      GO TO 173
320. 17      C      175 LDR=RG(HD)
321. 17      C      DR=DR+RG(HD)*RH(HD)
322. 17      C      GO TO 173
323. 17      C      176 LDR = RD(HD)
324. 17      C      DR=DR+RD(HD)*RH(HD)
325. 17      C      177 IF (ISPRO.LE.0) GO TO 180
326. 17      C      ND= IFIX(ID(N))
327. 17      C      IF (ND.EQ.0) GO TO 180
328. 17      C      IF (ELD(HD).GT.PHI(N)) GO TO 180
329. 17      C      UX = FLD(HD)
330. 17      C      DR = DR + FLD(HD)*ELD(HD)
331. 17      C
332. 17      C      — SIP REVERSE ALGORITHM —
333. 17      C      — FORWARD SUBSTITUTE, COMPUTING INTERMEDIATE VECTOR V —
334. 17      C      180 E = -B -D -F -H -SU -Z -RHO -XOR -ETOB
335. 17      C      IF (K.NE.K1) GO TO 181
336. 17      C      IF (PHI(NK).GE.BOTTOM(NMA)) GO TO 181
337. 17      C      IF (PHI(NKA).GE.BOTTOM(NMA)) DR=DR+SUBOTTOM(NMA)
338. 17      C      IF (PHI(NKA).GE.BOTTOM(NMA)) GO TO 179
339. 17      C      E=E+SU
340. 17      C      SU=0
341. 17      C      GO TO 181
342. 17      C      179 E=E+9
343. 17      C      181 IF (K.NE.K0) GO TO 183
344. 17      C      IF (PHI(NKB).GE.BOTTOM(NMA)) GO TO 183
345. 17      C      IF (PHI(N).GE.BOTTOM(NMA)) DR=DR+SUBOTTOM(NMA)
346. 17      C      IF (PHI(N).GE.BOTTOM(NMA)) GO TO 182
347. 17      C      E=E+2
348. 17      C      182 Z=0
349. 17      C      183 CONTINUE
350. 17      C      K=H/((EL(NKA)+GL(NKA)))
351. 17      C      D=(A/((EL(NKA)+GL(NKA))+GL(HJB)))
352. 17      C      C=BL*EL(NKA)
353. 17      C      D=C*FL(HJB)
354. 17      C      MU=CL*GL(HJB)
355. 17      C      U=BL*GL(HKA)
356. 17      C      IF (K.EQ.0) GO TO 190
357. 17      C      AL=(EL(NKA)+PL(NKA))/D
358. 17      C      MU=AL*EL(NKA)
359. 17      C      TUMAL=PL(NKA)
360. 17      C      DL=E*(A+C*H/((EL(NKA)+GL(NKA))+GL(HJB))-BL*GL(NKA))-C*EL(NKA)-CL*EL(HJB)
361. 17      C      EL(N)=(F-B*D*(C+A)/D)/DL
362. 17      C      FL(N)=(B-A*D*(TUMAL))/DL
363. 17      C      GL(N)=(Z-B*D*(UH)/D)/DL
364. 17      C      ZPHI=0.ED
365. 17      C      IF (K.NE.1) ZPHI=ZPHI(NKB)
366. 17      C      RESA=BPHI(NID)-BPHI(HJB)-EPHII(NJN)-EPHII(NJA)-EPHII(NIA)-SUBPHI(N
367. 17      C      1KA)-ZPHI-HELL(N)-RNHOLD(N)-ORETOD
368. 17      C      V(N)=(RES-ALEVI(NKA)-BLV(NKA)-CLV(NJB))/DL
369. 17      C      GO TO 200
370. 17      C      190 DL=E*(B-A*D*(C+A)/D)-BL*FL(NKA)-CL*EL(HJB)
371. 17      C      EL(N)=(F-B*D*(C+A)/D)/DL
372. 17      C      FL(N)=(B-A*D*(C+A)/D)/DL
373. 17      C      GL(N)=(Z-B*D*(UH)/D)/DL
374. 17      C      ZPHI=0.ED
375. 17      C      IF (K.NE.1) ZPHI=ZPHI(NKB)
376. 17      C      RESA=BPHI(NIB)-BPHI(HJB)-EPHII(NJA)-EPHII(NSA)-EPHII(NIA)-ZPHI-HEL
377. 17      C      LL(N)=RNHOLD(N)-ORETOD
378. 17      C      V(N)=(RES-BLV(NIA)-CLV(HJB))/DL
379. 17      C      200 CONTINUE
380. 17      C
381. 17      C      — BACK SUBSTITUTE FOR VECTOR XI —
382. 17      C      DO 210 K=1,K0
383. 17      C      DO 210 I=2,I1
384. 17      C      DO 210 J=1,J2
385. 17      C      J0=JO-J
386. 17      C      N=I+(J0-1)*JO+(K-1)*IJ
387. 17      C      IF (IT(1),ED,0,-DR,S(N),LT,0,) GO TO 210
388. 17      C      GLXI=0.ED
389. 17      C      IF (K.NE.1) GLXI=GL(N)*XI(N-IJ)
390. 17      C      XI(N)=V(N)-EL(N)*XI(N-IJ)-FL(N)*XI(N-1)-GLXI
391. 17      C
392. 17      C      — COMPARE MAGNITUDE OF CHANGE WITH CLOSURE CRITERIA —
393. 17      C      TCH=PARS(XI(1))
394. 17      C      IF (TCH.LE.BIO) GO TO 202
395. 17      C      BIO = TCH
396. 17      C      IXH = I
397. 17      C      JNH = JN
398. 17      C      KKH = K
399. 17      C      BIGS = XI(N)
400. 17      C      202 PCH=(XI(1)-XI(N))/BIGS
401. 17      C      CONTINUE
402. 17      C      PRINT 211,IIN,JIN,KKH,BIGS
403. 17      C      211 FORMAT (1X,J10.0,20.7)
404. 17      C      IF (BIGS.GT.ENS) TEST=1,
405. 17      C      TEST=(IT+1)*BIO
406. 17      C      IF (TEST.EQ.0,) RETURN
407. 17      C      GO TO 60
408. 17      C
409. 17      C      —FORMATS—
410. 17      C
411. 17      C
412. 17      C
413. 17      C      220 FORMAT ('/EXCEEDED PERMITTED NUMBER OF ITERATIONS//',3F15.7)
414. 17      C      230 FORMAT ('//IHO,T5,ZN ITERATION PARAMETERS: 6E15.7/(2BX,6E15.7)')

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415. 17 240 FORMAT ('-->44X,'SOLUTION BY THE STRONGLY IMPLICIT PROCEDURE',/4X,
416. 17 143(' ')
417. 17 END
1. 00 SUBROUTINE CDEF(PHI,SYRT,OLD,T,S,TR,TC,TK,WELL,DELY,DELT,FACT
2. 00 1,PERM,BOTTOM,ORE)
3. 00
4. 00 COMPUTE COEFFICIENTS
5. 00
6. 00
7. 00 SPECIFICATIONS:
8. 01 CHARACTER *4(ICHK,IEOH,IEVAP,IALT,INAPP,IMAPP
9. 00 CHARACTER *4(DRAU,IHEAD,IFLD,IK1,IK2,IWATER,IRE,IPU1,IPU2,ITK
10. 00 COMMON /INTEGR/ ID,JD,KO,II,J1,K1,I,J,X,NPER,KTH,ITMAX,LENGTH,RP,N
11. 00 IWD,HUNT,IFINAL,IT,KT,IHEAD,IDRAU,IFLD,IERR,IZ,J2,K2,IMAX,ITMAX,NC
12. 00 ZH,IDL,IWATER,IMAP,IP,JP,JD,IR,IR1,IR2,IPU1,IPU2,ITK,IND
13. 01 3,IEON,IEVAP,IALT,INAPP,IMAPP
14. 00 COMMON /SPARAM/ THAX,COLT,DEL,ERR,TEST,SUM,SIMP,OR,GET,ETDIST
15. 00 COMMON /SARRAY/ ICHK(14),LEVEL1(*),LEVEL2(*)
16. 01
17. 00
18. 00 DIMENSION PHI(10,JO,KO)* STRT(10,JO,KO), DLD(10,JO,KO), T(10,JO,KO
19. 00 ), S(10,JO,KO), TR(10,JO,KO), TC(10,JO,KO), TK(1K,J+K2), WELL(10,
20. 00 2,JO,KO), DELX(JO), DELY(10), DELZ(KO), FACT(KO,3), PERMIP,JP, BOT
21. 00 STOM(IP,JP),ORE(10,JO,KO)
22. 00 RETURN
23. 00
24. 00 *****COMPUTE TRANSMISSIVITY FOR UPPER HYDROLOGIC UNIT WHICH
25. 00 IT IS UNCONFINED*****
26. 00
27. 00 ENTRY TRANS(1K)
28. 00
29. 00 DO 10 I=2,11
30. 00 DO 10 J=2,11
31. 00 IF (PERM(I,J).EQ.0.) GO TO 10
32. 00 T(I,J,KO)=PERM(I,J)*(PHIX(I,J,KO)-BOTTOM(I,J))
33. 00 IF (T(I,J,KO).GT.0.) GO TO 10
34. 00 IF (WELL(I,J,KO).LT.0.) WRITE (6,60) I,J,KO
35. 00 IF (WELL(I,J,KO).GE.0.) WRITE (6,70) I,J,KO
36. 00 PERM(I,J)=0.
37. 00 T(I,J,KO)=0.
38. 00 TR(I,J,KO)=0.
39. 00 TC(I,J,KO)=0.
40. 00 TK(I,J,KO)=0.
41. 00 IF (KO.EQ.1) TK(I,J,K)=0.
42. 00 PHIX(I,J,KO)=1,E30
43. 00
44. 00 10 CONTINUE
45. 00 IF (KO.EQ.1) RETURN
46. 00
47. 00 NI=KO
48. 00 NC=KO
49. 00 NM=1
50. 00
51. 00 DO 20
52. 00 --COMPUTE T COEFFICIENTS--
53. 00
54. 00 ENTRY TCOF
55. 00
56. 00 NI=1
57. 00 NC=KO
58. 00 NM=1
59. 00
60. 00 20 DO 40 K=NI,N2
61. 00 DO 40 J=1,11
62. 00 DO 40 I=1,11
63. 00 IF (T(I,J,K).EQ.0.) GO TO 40
64. 00 IF (T(I,J,1).EQ.0.) GO TO 30
65. 00 TR(I,J,K)=(2.4*T(I,J-1,K)+T(I,J,K))/((T(I,J,K)*DELX(J+1)+T(I,J+1,K)*
66. 00 DELX(J)))*FACT(K+1)
67. 00 30 IF (T(I,J,K).EQ.0.) GO TO 40
68. 00 TC(I,J,K)=(2.4*T(I,J+1,K)+T(I,J,K))/((T(I,J,K)*DELY(I+1)+T(I+1,J,K)*
69. 00 DELY(I)))*FACT(K+2)
70. 00
71. 00 40 CONTINUE
72. 00 IF (KO.EQ.1,OR,ITK.EQ.1CHK(10)) RETURN
73. 00 DO 50 K=NC,N1
74. 00 DO 50 I=2,11
75. 00 DO 50 J=2,11
76. 00 IF (T(I,J,K+1).EQ.0.) GO TO 50
77. 00 T1=T(I,J,K)*FACT(K+1)
78. 00 T2=T(I,J,K+1)*FACT(K+1)
79. 00 TK(I,J,K)=(2.4*T1+T2)/(11*DELZ(K+1)+T2*DELZ(K))
80. 00
81. 00 50 CONTINUE
82. 00 RETURN
83. 00
84. 00
85. 00 60 FORMAT ('-->20(*'),'WELL',213,' IN LAYER',13,' GOES DRY',20(*')
86. 00 70 FORMAT ('-->20(*'),'NODE',213,' IN LAYER',13,' GOES DRY',20(*')
87. 00 END
88. 00 SUBROUTINE CHECK1(IFHI,STR,OLD,T,S,TR,TC,TK,WELL,DELY,DELT,FACT,
89. 00 1CT,IFLD,FLW,JD,FLD,ELD,IIR,IR1,IR2,ISPGD,IRIV,BOTTOM,ORE,2RCF)
90. 00
91. 00 COMPUTE A VOLUMETRIC BALANCE
92. 00
93. 00 SPECIFICATIONS:
94. 00 PERM,4,I,DR,IFLD
95. 00 CHARACTER *4(ICHK,IEOH,IEVAP,IALT,INAPP,IMAPP
96. 00 CHARACTER *4(DRAU,IHEAD,IFLD,IK1,IK2,IWATER,IRE,IPU1,IPU2,ITK,IND
97. 00 COMMON /INTEGR/ ID,JD,KO,II,J1,K1,I,J,X,NPER,KTH,ITMAX,LENGTH,RP,N
98. 00 IWD,HUNT,IFINAL,IT,KT,IHEAD,IDRAU,IFLD,IERR,IZ,J2,K2,IMAX,ITMAX,NC
99. 00 ZH,IDL,IWATER,IMAP,IP,JP,JD,IR,IR1,IR2,IPU1,IPU2,ITK,IND
100. 01 3,IEON,IEVAP,IALT,INAPP,IMAPP
101. 00 COMMON /SPARAM/ THAX,COLT,DEL,ERR,TEST,SUM,SIMP,OR,GET,ETDIST
102. 00 COMMON /SARRAY/ ICHK(14),LEVEL1(*),LEVEL2(*)
103. 01
104. 00 COMMON /CV/ ETFLXT,STR1,ORE,CS1,CHDT,RFLX1,PUMP1,CSFLNT,SPFLX
105. 00
106. 00 DIMENSION PHI(10,JO,KO), STRT(10,JO,KO), DLD(10,JO,KO), T(10,JO,KO
107. 00 ), S(10,JO,KO), TR(10,JO,KO), TC(10,JO,KO), TK(1K,J+K2), WELL(10,
108. 00 2,JO,KO), DELX(JO), DELY(10), DELZ(KO), FACT(KO,3), FL0(NCH,3), FL0
109. 00 3(NCH),ORE(10,JO,KO),CH01(B),CH02(B),OREFLX(B),PUMP(B),
110. 00 4, ID(10,JO,KO),FLD1(B),IDR1(B),JD(KO),PM(1?)*C11?*R(B),
111. 00 5,FLFLX(B),SFDR(B),FLUX(B),RFLUX(B),FLXN(B),BOTTOM(IP,JP)
112. 00 6,INDEX1(*),INDEX2(*),INDEX3(*),INDEX4(*),INDEX5(*),INDEX6(*)

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28.      19      7.GRNDI(10-J0),RCF(10-J0,K0)
29.      19      RETURN
30.      C      -----
31.      19      ENTRY CHECK
32.      19      -----
33.      19      ---INITIALIZE VARIABLES---
34.      19      DO 5 K=1,K0
35.      19      S0TR(K)=0
36.      19      CFLUX(K)=0
37.      19      PUMP(K)=0
38.      19      QREFLX(K)=0
39.      19      CHD1(K)=0
40.      19      CHD2(K)=0
41.      19      FLUX(K)=0
42.      19      RFLUX(K)=0
43.      19      FLX(K)=0
44.      19      ETFLUX(K)=0
45.      19      CONTINUE
46.      19      KK=0
47.      19      C1TR=0
48.      19      R1TR=0
49.      19      FLUDS=0
50.      19      PLUTR=0
51.      19      S0TR=0
52.      19      QRETR=0
53.      19      CHCTR=0
54.      19      CHSTR=0
55.      19      PUMTR=0
56.      19      CFLXR=0
57.      19      II=0
58.      19      -----
59.      19      ---NO MASS ACCUMULATIONS FROM PREVIOUS PUMPING PERIOD---
60.      19      IF (1NAPP,NE,1ONK(15)) GO TO 8
61.      19      ETFLXT=0
62.      19      RELXT=0
63.      19      SELXT=0
64.      19      START=0
65.      19      QRET=0
66.      19      CHDT=0
67.      19      CIST=0
68.      19      PUMT=0
69.      19      CFLUXT=0
70.      19      SUPR=0
71.      19      -----
72.      19      C      -----
73.      19      8 DO 150 K=1,K0
74.      19      DO 150 I=2,II
75.      19      DO 150 J=2,J1
76.      19      IF (ITL(I,J,K),EQ,0,) GO TO 120
77.      19      AREA=DELY(I,J)*DELZ(I)
78.      19      VOLUME=AREA*DELZ(I)
79.      19      IF (K,NE,K0,OR,1D(I-J-K),EQ,0,) GO TO 9
80.      19      DN=PHE(I,J,K-1)
81.      19      IF (PHE(I,J,K-1),LT,BOTTOM(I,J)) DN=BOTTOM(I,J)
82.      19      X=(PHE(I,J,K)-DN)*TK(I,J,K-1)*AREA
83.      19      XK=HCK+1
84.      19      INDEXX(XK)=I
85.      19      INDEXC(XK)=J
86.      19      DFLUX(XK)=X
87.      19      CONTINUE
88.      19      IF (S(I-J-K),EQ,0,) GO TO 180
89.      19      C      -----
90.      19      9 COMPUTE FLOW RATES TO AND FROM CONSTANT HEAD BOUNDARIES--- 00011810
91.      19      90011820
92.      19      90011830
93.      19      II=II+1
94.      19      JFLU(I,I)=0
95.      19      JFLU(I,I-1)=K
96.      19      JFLU(I,I-2)=I
97.      19      JFLU(I,I-3)=J
98.      19      IF (S(I,J-1,K),LT,0.,OR,T(I,J-1,K),EQ,0,.) GO TO 30
99.      19      X=(PHE(I,J,K)-PHE(I,J-1,K))/DELZ(I)*TR(I,J,K)
100.     19      IF (ED,NE,1ONK(11)) X=X*DELZ(K)
101.     19      JFLU(I,I)=JFLU(I,I)+X
102.     19      IF (X,10,-30,20) 00012000
103.     19      10 CHD1(K)=CHD1(K)+X 00012100
104.     19      GO TO 30 00012110
105.     19      20 CHD2(K)=CHD2(K)+X
106.     19      30 IF (S(I,J-1,K),LT,0.,OR,T(I,J-1,K),EQ,0,.) GO TO 60
107.     19      X=(PHE(I,J,K)-PHE(I,J-1,K))/DELZ(I)*TR(I,J,K)
108.     19      IF (ED,NE,1ONK(11)) X=X*DELZ(K)
109.     19      JFLU(I,I)=JFLU(I,I)+X
110.     19      IF (X,40,-60,50) 00012140
111.     19      40 CHD1(K)=CHD1(K)+X 00012170
112.     19      GO TO 40 00012230
113.     19      50 CHD2(K)=CHD2(K)+X 00012190
114.     19      60 IF (K,NE,1) GO TO 90 00012290
115.     19      11 IF (S(I,J,K-1),LT,0.,OR,T(I,J,K-1),EQ,0,.) GO TO 90 00012210
116.     19      X=(PHE(I,J,K)-PHE(I,J,K-1))/DELZ(I)*AREA
117.     19      IF (ED,NE,1ONK(11)) X=X*DELZ(K)
118.     19      X=(PHE(I,J,K)-PHE(I,J,K-1))/DELZ(I)*AREA
119.     19      FLOW(I,I)=FLOW(I,I)+X 00012260
120.     19      IF (X,70,-50,80) 00012270
121.     19      70 CHD1(K)=CHD1(K)+X 00012290
122.     19      GO TO 70 00012390
123.     19      80 CHD2(K)=CHD2(K)+X 00012300
124.     19      90 IF (K,NE,K) GO TO 120 00012310
125.     19      IF (S(I,J,K+1),LT,0.,OR,T(I,J,K+1),EQ,0,.) GO TO 120 00012320
126.     19      UP=PHE(I,J,K)
127.     19      IF (K,NE,K0-1,AND,PHE(I,J,K),LT,BOTTOM(I,J)) UP=BOTTOM(I,J)
128.     19      X=(PHE(I,J,K)-PHE(I,J,K-1))/TK(I,J,K)*AREA
129.     19      JFLU(I,I)=JFLU(I,I)+X
130.     19      IF (X,100,-120,110) 00012380
131.     19      100 CHD1(K)=CHD1(K)+X 00012400
132.     19      GO TO 120 00012410
133.     19      110 CHD2(K)=CHD2(K)+X 00012420
134.     19      120 IF (S(I-1,J,K),LT,0.,OR,T(I-1,J,K),EQ,0,.) GO TO 150 00012430
135.     19      X=(PHE(I,J,K)-PHE(I-1,J,K))/TK(I-1,J,K)*DELX(J)
136.     19      IF (ED,NE,1ONK(11)) X=X*DELZ(K)
137.     19      FLOW(I,I)=FLOW(I,I)+X
138.     19      IF (X,130,-150,140) 00012460
139.     19      130 CHD1(K)=CHD1(K)+X 00012470

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140.      19      DO TO 150                                         00012480
141.      19      140  CH02(K)=CH02(K)+X
142.      19      150  IF (SI(I+1,J,K).LT.0.,OR,T(I+1,J,K).EQ.0.) GO TO 220  00012490
143.      19      X*(PH(I+1,J,K))-PH(I+1,J,K)*T(I,J,K)*DELX(J)
144.      19      IF (TENM.EQ.TOM(SI(I))) X=X*DEL2(K)
145.      19      FLOW(I)=FLQU(X)
146.      19      IF (X)>140. ZD(120)
147.      19      160  CH01(K)=CH01(K)+X
148.      19      DO TO 220                                         00012510
149.      19      170  CH02(K)=CH02(K)+X
150.      19      DO TO 220                                         00012530
151.      19      C   ---CHECK FOR EQUATION BEING SOLVED---
152.      19      180  IF (IEQN.EQ.ICHK(1)) GO TO 213
153.      19
154.      19      C   ---EQUATION 4---
155.      19      C   ---RECHARGE AND WELLS---
156.      19      IF (IRE.EQ.ICHK(7)) OREFLX(X)=OREFLX(X)+ORE(I,J,K)*AREA  00012550
157.      19      IF (WELL(I,J,K).NE.0) ZD(210*190)
158.      19      190  PUMP(K)=PUMP(K)+WELL(I,J,K)*AREA-RCF(I,J,K)
159.      19      CFLUX(K)=CFLUX(K)-RCF(I,J,K)
160.      19
161.      19      C   ---COMPUTE VOLUME FROM STORAGE---
162.      19      210  STOR(K)=STOR(K)+SI(I,J,K)*(OLD(I,J,K)-PHI(I,J,K))*AREA  00012560
163.      19      GO TO 207                                         00012570
164.      19
165.      19      C   ---EQUATION 3---
166.      19      C   ---RECHARGE AND WELLS---
167.      19      213  IF (IRE.EQ.ICHK(7)) OREFLX(X)=OREFLX(X)+ORE(I,J,K)*VOLUME
168.      19      IF (WELL(I,J,K).NE.0) ZD(214*214*214)
169.      19      214  PUMP(K)=PUMP(K)+(WELL(I,J,K)*VOLUME-RCF(I,J,K))
170.      19      CFLUX(K)=CFLUX(K)+RCF(I,J,K)
171.      19
172.      19      C   ---COMPUTE VOLUME FROM STORAGE
173.      19      216  STOR(K)=STOR(K)+SI(I,J,K)*(OLD(I,J,K)-PHI(I,J,K))*VOLUME
174.      19      209  IF (K.NE.K0.OR.IEAPM.NE.ICHK(12)) GO TO 217
175.      19
176.      19      C   ---COMPUTE ET RATE---
177.      19      IF (PH(I+1,J,K).GE.OISMB(I,J)-STD191) GO TO 208
178.      19      ET0=0.0
179.      19      DO TO 217
180.      19      208  IF (PH(I+1,J,K).LE.OISMB(I,J)) GO TO 210
181.      19      ET0=0.0
182.      19      DO TO 217
183.      19      210  ET0=NET/ETDIST(PHI(I,J,K))-ETDIST-GRND(I,J)
184.      19      211  ETFLUX(K)=ETFLUX(K)-ETONPAZ
185.      19      212  IF (IRAV.LE.0) GO TO 212
186.      19
187.      19      C   ---COMPUTE LEAKAGE TO RIVER---
188.      19      HD = IFIX(I0(I,J,K))
189.      19      IF (HD.EQ.0) GO TO 222
190.      19      IF (PH(I,J,K).GT.RIVHD(HD)) GO TO 211                                         00012710
191.      19      RFLUX(K)=RFLUX(K)+PH(HD)-RSHD(HD)*AREA*RFLUX(K)
192.      19      GO TO 212                                         00012720
193.      19      211  RFLUX(K)=(PH(HD)-PH(I,J,K))/AREAHDRD(HD)*RFLUX(K)
194.      19      212  IF (ISPRO.LE.0) GO TO 222                                         00012740
195.      19
196.      19      C   ---COMPUTE LEAKAGE TO SPRING---
197.      19      HD = IFIX(I0(I,J,K))
198.      19      IF (HD.EQ.0) GO TO 220
199.      19      IF (RIVHD(HD).GT.PHS(I,J,K)) GO TO 220                                         00012750
200.      19      RFLUX(K)=FLUX(K)+FLD(HD)*AREAHDRD(HD) - STR(I,J,K)
201.      19      FLUX(K)=FLX(K)+FLD(HD)*AREAHDRD(HD) - PHI(I,J,K)
202.      19      CONTINUE                                         00012760
203.      19
204.      19      C   ---COMPUTE CUMULATIVE VOLUMES, TOTALS, AND DIFFERENCES---  00012780
205.      19
206.      19      FLXPT=0.
207.      19      DO 221 K=1,K0                                         00012860
208.      19      221  ETFLXT=ETFLXT-ETFLUX(K)*DELT
209.      19      ETTR=ETTR+CFLUX(K)
210.      19      RFLUX=RFLXT-RFLUX(K)*DELT
211.      19      RFLTR=RFLTR-RFLUX(K)
212.      19      FLUX = FLUX(K) + FLDX
213.      19      SFLXT = SFLXT - FLUX(K)*DELT                                         00012870
214.      19      FLXT = FLUX(K) + FLUXTR                                         00012870
215.      19      STOR = STOR + STOR(K)
216.      19      STORTR = STORTR + STOR(K)                                         00012880
217.      19      ORET = ORET + OREFLX(K)*DELT
218.      19      DRET = DRET + OREFLX(K)                                         00012890
219.      19      CHDT = CHDT + CH01(K)*DELT
220.      19      CHOTR = CHOTR + CH01(K)                                         00012900
221.      19      CHOT = CHOT + CH02(K)*DELT
222.      19      CSTR = CSTR + CH02(K)                                         00012910
223.      19      PUMPT = PUMPT - PUMP(K)*DELT
224.      19      PUMPTR = PUMPTR - PUMP(K)
225.      19      CFLUXTR = CFLUXTR + CFLUX(K)*DELT
226.      19      CFLXT = CFLXT + CFLUX(K)*DELT                                         00012940
227.      19      221  CFLXT=CFLXT+CFLUX(K)
228.      19      TOTL1=STOR+ORET+CFLXT+CHST+FLXPT
229.      19      TOTL2=CHDT+PUMPT+ETFLXT+SFLXT+RFLXT
230.      19      SURR = ORET + CFLXT+CHDT+DRET+PUMPTR+ETTR + FLUX *  00013090
231.      19      221  ISOTR=RFLTR
232.      19      DIFF=TOTL2-TOTL1                                         00013110
233.      19      PERCT=0.0                                         00013120
234.      19      IF (TOTL2.EQ.0.) GO TO 230
235.      19      PERCT=DIFF/TOTL2*100.
236.      19      230  RETURN                                         00013130
237.      19
238.      19      C   ---PRINT RESULTS---
239.      19      238  WRITE(6,260) STORTR, ORET, CFLXT, ORET, PUMPTR, CFLUXTR, 00013140
240.      19      RFLXT, PUMPT, SURR, SFLXT, ETFLXT, TOTL2, DIFF, PERCT
241.      19      240  WRITE(6,200)
242.      19      ENTRY CURITE                                         00013150
243.      19      242  WRITE(6,200)
244.      19      WRITE(6,260) K, CH01(K), CH02(K), OREFLX(K), PUMP(K), 00013210
245.      19      CFLUX(K), STOR(K), RFLUX(K), FLUX(K), ETFLUX(K)
246.      19

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19 CONTINUE
20 FORMAT(6X,LAYER,I3,BN, DCHOT=,011.5,7H CHIN=,011.5,6H DRE=,011.5,7H
21 ,6H PMP=,011.5,6H QLUX=,011.5,7H STOR=,011.5,7H,0X/RIVER=,
22 ,011.5,7H SPRIN=,011.5,7H ET=,011.5,7)
23 IF (NCH.EQ.9) GO TO 240
24 WRITE(6,270)
25 WRITE(6,290) ((IFIX(JFLQ(I,J)),J=1,3),FLQ(I)+I-1,NCH)
26
27 C   ----COMPUTE VERTICAL FLOW-----
28 240 X=0.
29 Y=0.
30 IF (X0.EQ.1) RETURN
31 DO 250 I=1,1
32 DO 250 J=1,1
33 X=X+(PHI(I,J)-PHI(I,J-1))*TK(I,J)*DELX(J)*DELY(I)
34 HZ=PHI(I,J,K1)
35 IF(PHI(I,J,K1).LT.BOTTOM(I,J)) HZ= BOTTOM(I,J)
36 IF(PHI(I,J,K1).LT.BOTTOM(I,J).AND.PHI(I,J,K0).LT.BOTTOM(I,J)) GO
37 ITO 250
38
39 T=-(HZ-PHI(I,J,K0))/TK(I,J,K1)*DELX(J)*DELY(I)
40
41 CONTINUE
42 WRITE(6,290) Y,X
43 DO 430 I=1,NX
44 WRITE(6,32) INDEX(I),INDEX(I),DFLUX(I)
45 FORMAT(7X,BN SPRIN >210,1E15,7)
46 SPLX=0.0
47 DO 44 I=1,NX
48 SPLX=SPLX+DFLUX(I)
49 WRITE(6,44) SPLX
50 FORMAT(7X,3BN TOTAL SPRING LEAKAGE FOR 2ND LAYER >1E15,7)
51 RETURN
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52.      NC=1;N3=1;R=101/2
53.      ND=1;C=1;NR
54.      NE=MAX(1,JS,NB)
55.      VF1(I3)=D10IT(ND)
56.      VF2(I3)=D10IT(ND)
57.      VF3(I3)=D10IT(ND)
58.      YLABEL(3)=MEASUR
59.      YLABEL(6)=ESUR
60.      DO 40 I=1,NE
61.      NDX=NE-I
62.      NY=Y-T-1
63.      IF (NY.GE.R) GO TO 50
64.      TN(I)=SFANNY/SCALE
65.      IF (NMX.LY.0) GO TO 60
66.      NT(I)=SFANNY/SCALE
67.      40 CONTINUE
68.      RETURN
69.      ****
70.      C   ENTRY PRNTA(H,I,LA)
71.      C   SUBROUTINE PRNTA(H,I,LA)
72.      C   —VARIABLES INITIALIZED EACH TIME A PLOT IS REQUESTED—
73.      DIST=WIDTH+DELY((J1)/2)
74.      J1=J1
75.      LL=1
76.      Z=H0000SF
77.      IF (NB.EQ.1) WRITE (6,300) (TITLE(I),I=1,3)/LA
78.      IF (NB.EQ.2) WRITE (6,300) (TITLE(I),I=4,6)/LA
79.      DO 290 I=1,NA
80.      C   —LOCATE X AXES—
81.      IF ((I.EQ.1).OR.(I.EQ.NA)) GO TO 70
82.      PRNT(I1)=SYM(12)
83.      PRNT(I2)=SYM(13)
84.      IF (((I-1)/NMX).NE.I-1) GO TO 90
85.      PRNT(I1)=SYM(14)
86.      PRNT(NB)=SYM(14)
87.      GO TO 90
88.      C   —LOCATE Y AXES—
89.      70 DO 80 J=1,NS
90.      IF ((J-1)/NMX2.EQ.J-1) PRNT(J)=SYM(14)
91.      80 IF ((J-1)/NMXC.NC.J-1) PRNT(J)=SYM(13)
92.      C   —COMPUTE LOCATION OF NODES AND DETERMINE APPROPRIATE SYMBOL—
93.      90 IF (DIST.LT.0..OR.DIST.LT.Z-XH1*SF) GO TO 240
94.      YLEN=DELY((J1)/2)
95.      DO 220 L=2,LL
96.      JYL=EN(LC)*YSF+1.5
97.      IF ((YL,JYL,LA)>20,0,) GO TO 160
98.      IF ((YL,JYL,LA).LT.0,) GO TO 210
99.      INDEX=L
100.     GO TO 100
101.     K=(SYM(11,JYL,LA)-PRNT(J1))/FACT1
102.     —TO CYCLE SYMBOLS FOR DRAWDOWN: REMOVE C FROM COL. 1 OF NEXT CARD-
103.     K=NOD(K,10.)
104.     GO TO 120
105.     110 K=SYM(11,JYL,LA)/FACT2
106.     120 IF (K) 130,140,150
107.     130 IF (J>2,GT,0) PRNT(J-2)=SYM(13)
108.     H=H+1.5
109.     IF (H.LT.100) GO TO 150
110.     GO TO 140
111.     140 H=K+.5
112.     IF (H.LT.100) GO TO 150
113.     IF (H.GT.777) GO TO 190
114.     INDEX1=H/100
115.     IF (J>2,GT,0) PRNT(J-2)=SYM(INDX3)
116.     H=H-INDEX1
117.     GO TO 140
118.     150 INDEX1=MOD(H,10)
119.     IF (INDEX1,ED,0) INDEX1=10
120.     —TO CYCLE SYMBOLS FOR DRAWDOWN: REMOVE C FROM COL. 1 OF NEXT CARD-
121.     IF (NOD,1,ED,0) GO TO 170
122.     INDEX2=H/10
123.     IF (INDEX2,ED,0) INDEX2=15
124.     GO TO 160
125.     160 INDEX1=15
126.     INDEX2=15
127.     180 IF ((J-1,GT,0) PRNT(J-1)=SYM(INDX2)
128.     PRNT(J)=SYM(INDX1))
129.     GO TO 220
130.     190 DO 200 I1=1,3
131.     J1=J-2+II
132.     200 IF ((J1,GT,0) PRNT(J1)=SYM(31))
133.     IF ((SIL,J1,LA).LT.0,) PRNT(J1)=SYM(14)
134.     210 YLEN=YLEN+(DELY(L1)+DELY(L+1))/2.
135.     DIST=DIST-(DELX(JJ)+(DELX(JJ-1))/2.
136.     JJ=JJ-1
137.     IF (JJ,ED,0) GO TO 240
138.     IF (DIST.GT.2*XH1*SF) GO TO 230
139.     240 CONTINUE
140.     C   —PRINT AXES, LABELS, AND SYMBOLS—
141.     IF ((I-NALL).ED,0) GO TO 260
142.     IF ((I-1)/NMX1-(I-1)) 270,250,270
143.     250 ((I1A+1)-(I-1)/4
144.     WRITE(6,VF1) (BLANK(J),J=1,NC),(PRNT(J),J=1,NB),DN(I114)
145.     GO TO 280
146.     260 WRITE(6,VF2) (BLANK(J),J=1,NC),(PRNT(J),J=1,NB),YLABL(LL)
147.     LL=LL+1
148.     GO TO 280
149.     270 WRITE(6,VF2) (BLANK(J),J=1,NC),(PRNT(J),J=1,NB)
150.     C   —COMPUTE NEW VALUE FOR Z AND INITIALIZE PRNT—
151.     280 Z=z-2.*XH1*SF
152.     DO 290 J=1,NS
153.     290 PRNT(J)=SYM(15)
154.     C   —NUMBER AND LABEL Y AXIS AND PRINT LEGEND—

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166.      88      WRITE (6,WF3) 'BLANK(J),J=1,NC),(TN(I),I=1,NQ)
167.      89      WRITE (6,329) (YLABEL(I),I=1,6)
168.      90      IF (NQ.EQ.1) WRITE (6,310) FACT1
169.      91      IF (NQ.EQ.2) WRITE (6,310) FACT2
170.      92      RETURN
171.      93      C
172.      94      C
173.      95      C
174.      96      C
175.      97      C
176.      98      C
177.      99      C
178.      100      C
179.      101      C
180.      102      C
181.      103      C
182.      104      C
183.      105      C
184.      106      C
1.      107      C
2.      108      C
3.      109      C
4.      110      C
5.      111      C
6.      112      C
7.      113      C
8.      114      C
9.      115      C
10.      116      C
11.      117      C
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18.      124      C
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28.      134      C
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30.      136      C
31.      137      C
32.      138      C
33.      139      C
34.      140      C
35.      141      C
36.      142      C
37.      143      C
38.      144      C
39.      145      C
40.      146      C
41.      147      C
42.      148      C
43.      149      C
44.      150      C
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46.      152      C
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110.      216      C
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840.      946      C
841.      947      C
842.      948      C
843.      949      C
844.      950      C
845.      951      C
846.      952      C
847.      953      C
848.      954      C
849.      955      C
850.      956      C
851.      957      C
852.      958      C
853.      959      C
854.      960      C
855.      961      C
856.      962      C
857.      963      C
858.      964      C
859.      965      C
860.      966      C
861.      967      C
862.      968      C
863.      969      C
864.      970      C
865.      971      C
866.      972      C
867.      973      C
868.      974      C
869.      975      C
870.      976      C
871.      977      C
872.      978      C
873.      979      C
874.      980      C
875.      981      C
876.      982      C
877.      983      C
878.      984      C
879.      985      C
880.      986      C
881.      987      C
882.      988      C
883.      989      C
884.      990      C
885.      991      C
886.      992      C
887.      993      C
888.      994      C
889.      995      C
890.      996      C
891.      997      C
892.      998      C
893.      999      C
894.      990      C
895.      991      C
896.      992      C
897.      993      C
898.      994      C
899.      995      C
900.      996      C
901.      997      C
902.      998      C
903.      999      C
904.      990      C
905.      991      C
906.      992      C
907.      993      C
908.      994      C
909.      995      C
910.      996      C
911.      997      C
912.      998      C
913.      999      C
914.      990      C
915.      991      C
916.      992      C
917.      993      C
918.      994      C
919.      995      C
920.      996      C
921.      997      C
922.      998      C
923.      999      C
924.      990      C
925.      991      C
926.      992      C
927.      993      C
928.      994      C
929.      995      C
930.      996      C
931.      997      C
932.      
```

Appendix B

Program Input Data Documentation

DATA DECK INSTRUCTIONS

Group I: Title, Simulation Options and Problem Dimensions

This group of cards, which are read by the main program, contains data required to dimension the model. To specify an option on card 4 punch the characters underlined in the definition. For an option not used, that section of card 4 can be left blank.

Note: Default typing of variables applies for all data input.

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1	1-80	20A4	HEADING	Any title the user wishes to print on one line at the start of output.
2	1-52	13A4	"	
3	1-10	I10	I0	Number of rows
	11-20	I10	J0	Number of columns
	21-30	I10	K0	Number of layers
	31-40	I10	ITMAX	Maximum number of iterations per time step
	41-50	I10	NCH	Number of constant head nodes
	51-60	I10	NSP	Number of spring nodes
	61-70	I10	NRIV	Number of river nodes
4	1-4	A4	IDRAW	<u>DRAW</u> to print drawdown
	6-9	A4	IHEAD	<u>HEAD</u> to print hydraulic head
	11-14	A4	IFLO	<u>MASS</u> to compute a mass balance
	16-18	A3	IDK1	<u>DK1</u> to read initial head, elapsed time, and mass balance parameters from unit 4 on disk

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
4	21-23	A3	IDK2	<u>DK2</u> to write computed head, elapsed time, and mass balance parameters on unit 4 (disk)
	26-29	A4	IWATER	<u>WATE</u> if the upper hydrologic unit is unconfined
	31-34	A4	IQRE	<u>RECH</u> for a constant recharge that may be a function of space
	36-39	A4	IPUL	<u>PUN1</u> to read initial head, elapsed time, and mass balance parameters from cards
	41-44	A4	IPU2	<u>PUN2</u> to punch computed head, elapsed time, and mass balance parameters on cards
	46-49	A4	ITK	<u>ITKR</u> to read the value of TK(I,J,K) for simulation in which confining layers are not represented by layers of nodes. $TK(I,J,K) = K_{zz}/b$
	51-54	A4	IEQN	<u>EQN3</u> if equation 3 (page 3, USGS Open File Report 75-438) is being solved; otherwise it is assumed that equation 4 is being solved (to represent a hydraulic unit by one layer of nodes).
	56-59	A4	IEVAP	<u>EVAP</u> to permit discharge by evapo-transpiration.
	61-63	A3	IALT	<u>ALT</u> to have alternative method for well pumping rates (modified for Cache Valley Model), besides the initial method.
	66-69	A4	IDNPP	<u>DNPP</u> to compute drawdown between each pumping period.
	71-74	A4	IMAPP	<u>MAPP</u> to compute mass balance for each pumping period.

Group II: Scalar Parameters

The parameters required in every problem are underlined. The other parameters are required as noted; when not required, their location on the card can be left blank. The G format is used to read E, F and I format data. Minimize mistakes by always right-justifying data in the field. If F format data do not contain significant figures to the right of the decimal point, the decimal point can be omitted.

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1	1-10	G10.0	<u>NPER</u>	Number of pumping periods for the simulation
	11-20	G10.0	<u>KTH</u>	Number of time steps between print-outs
			<u>Note:</u>	To print only the results for the final time step in a pumping period, make KTH greater than the expected number of time steps. The program always prints the results for the final time step.
	21-30	G10.0	<u>ERR</u>	Error criteria for closure (L)
			<u>Note:</u>	When the head change at all nodes on subsequent iterations is less than this value (for example, 0.01 foot), the program has converged to a solution for the time step.
	31-40	G10.0	<u>LENGTH</u>	Number of iteration parameters
	41-50	G10.0	<u>QET</u>	Maximum evapotranspiration rate (L/T)
	51-60	G10.0	<u>ETDIST</u>	Depth at which evapotranspiration ceases below land surface (L)
2	1-10	G10.0	<u>XSCALE</u>	Factor to convert model length unit to unit used in X direction on maps (e.g. to convert from feet to miles, XSCALE = 5280)
				<u>For no maps, card 2 is blank</u>
	11-20	G10.0	<u>YSCALE</u>	Factor to convert model length unit to unit used in Y direction on maps.
	21-30	G10.0	<u>DINCH</u>	Number of map units per inch
	31-40	G10.0	<u>FACT1</u>	Factor to adjust value of drawdown printed*
	41-49	9I1	<u>LEVEL1(I)</u>	Layers for which drawdown maps are to be printed. List the layers starting in column 41; the first zero entry terminates the printing of drawdown maps.

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
2	51-60	G10.0	FACT2	Factor to adjust value of head printed*
	61-69	9I1	LEVEL2(I)	Layers for which head maps are to be printed. List layers starting in column 61; the first zero entry terminates the printing of head maps.
	71-78	A8	MESUR	Name of map length unit
3	1-20	G20.10	SUM	
	21-40	G20.10	SUMP	
	41-60	G20.10	PUMPT	
	61-80	G20.10	CFLUXT	
4	1-20	G20.10	QRET	
	21-40	G20.10	CHST	
	41-60	G20.10	CHDT	
	61-80	G20.10	RVFLXT	Parameters in which elapsed time and cumulative volumes for mass balance are stored. For the start of a simulation insert three blank cards. <u>For continuation</u> of a previous run using cards as input, replace the three blank cards with the first three cards of punched output from the previous run. Using data from disk for input, leave the three blank cards in the data deck.
5	1-20	G20.10	STORT	
	21-40	G20.10	ETFLXT	
	41-60	G20.10	SFLXT	

<u>*Value of drawdown or head</u>	<u>FACT 1 or FACT 2</u>	<u>Printed Value</u>
	0.01	1
	0.1	5
52.57	1.0	53
	10.0	526
	100.0	***

Group III: Array Data

Each of the following data sets (except data set 1) consists of a parameter card and, if the data set contains variable data, a set of data cards. If the data set requires data for each layer, a parameter card and data cards (for layers with variable data) are required for each layer. Each parameter card contains at least five variables:

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
Every Parameter Card	1-10	G10.0	FAC	If IVAR = 0, FAC is the value assigned to every element of the matrix for this layer.
	11-20	G10.0	IVAR	If IVAR = 1, FAC is the multiplication factor for the following set of data cards for this layer.
	21-30	G10.0	IPRN	= 0 if no data cards are to be read for this layer. = 1 if data cards for this layer follow.
				= 0 if input data for this layer are to be printed. = 1 if input data for the layer are <u>not</u> to be printed.

Transmissivity Parameter Cards also have these Variables	31-40	G10.0	FACT(K,1)	Multiplication factor for transmissivity in x direction
	41-50	G10.0	FACT(K,2)	Multiplication factor for transmissivity in y direction
	51-60	G10.0	FACT(K,3)	Multiplication factor for hydraulic conductivity in the z direction. (Not used when confining bed nodes are eliminated and TK values are read)

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
Every Parameter Card	61-70	G10.0	IRECS	= 0 if the matrix is being read from cards or if each element is being set equal to FAC.
				= 1 if the matrix is to be read from disk (unit 2).
	71-80	G10.0	IRECD	= 0 if the matrix is <u>not</u> to be stored on disk.
				= 1 if the matrix being read from cards or set equal to FAC is to be stored on disk (unit 2) for later retrieval.

When data cards are included, start each row on a new card. To prepare a set of data cards for an array that is a function of space, the general procedure is to overlay the finite-difference grid on a contoured map of the parameter and record the average value of the parameter for each finite-difference block on coding forms according to the appropriate format. In general, record only significant digits and no decimal points (except for data set 2); use the multiplication factor to convert the data to their appropriate values. For example, if DELX ranges from 1000 to 15000 feet, coded values should range from 1-15; the multiplication factor (FAC) would be 1000.

<u>DATA SET</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1	1-80	8F 10.4	PHI(I,J,K)	Head values for continuation of a previous run (L)

Note: For a new simulation this data set is omitted. Do not include a parameter card with this data set.

2	1-80	8F 10.4	STRT(I,J,K)	Starting head matrix (L)
3	1-80	20F 4.0	S (I,J,K)	Storage coefficient (dimensionless)

Note: This matrix is also used to locate constant head boundaries by coding a negative number at constant head nodes. At these nodes T must be greater than zero. If equation 3 is to be solved, read specific storage instead of storage coefficient.

4	1-80	10G 8.0	T(I,J,K)	Transmissivity (L^2/t)
---	------	---------	----------	----------------------------

- Note 1) Zero values are required around the perimeter of the T matrix for each layer for reasons inherent in the computational scheme. This is done automatically by the program.
- 2) See the previous page for the additional requirements on the parameter cards for this data set.

- 3) If the upper active layer is unconfined and PERM and BOTTOM are to be read for this layer, insert a parameter card for this layer with only the values for FACT on it. If equation 3 is to be solved read hydraulic conductivity instead of transmissivity.

<u>DATA SET</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
5	1-80	20F 4.0	TK(I,J,K)	K _{zz} /b
Note: This data set is read only if specified in the options. The number of layers of TK values = K' - 1. See the discussion of the treatment of confining layers.				
6	1-80	20F 4.0	PERM(I,J)	Hydraulic conductivity (L/T) (see note 1 for data set 4)
7	1-80	20F 4.0	BOTTOM(I,J)	Elevation of bottom of water-table unit (L)
Note: Data sets 6 and 7 are required only for simulating unconfined conditions in the upper hydrologic unit.				
8	1-80	20F 4.0	QRE(I,J,K)	Recharge rate (L/2)
Note: Omit if not used				
9	1-80	8F 10.4	GRND(I,J)	Ground surface elevation
10	1-80	8G 10.0	DELX(J)	Grid spacing in x direction (L)
11	1-80	8G 10.0	DELY(I)	Grid spacing in y direction (L)
12	1-80	8G 10.0	DELZ(K)	Grid spacing in z direction (L)

Group IV: Spring Node Data Deck

This data deck contains arrays indicating the nodes which have springs and specifying the leakage factor and elevation of spring in each node.

<u>ARRAY</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>DEFINITION</u>
ID(I,J,K)	I-80	80G 1.0	Indicator array for cells containing a spring node, 1 for spring, 0 for no spring.
FLD(ND)	I-80	40F 2.0	Leakage factor for each spring node
ELD(ND)	I-80	20F 4.0	Elevation for each spring node

The FLD, ELD arrays each preceded by a multiplication factor (FAC) card.

<u>VARIABLE</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>DEFINITION</u>
FAC	I-10	G 10.0	The multiplication factor for the following set of data cards

Group V: River Node Data Deck

This data deck contains arrays indicating the nodes which have rivers and specifying the leakage factor and elevation of streambed and elevation of the bottom of the streambed layer.

<u>ARRAY</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>DEFINITION</u>
IDR(I,J,K)	1-80	80G 1.0	Indicator array for cells containing a river node, 1 for river, 0 for no river
RH(NRIV)	1-80	20F 4.0	River water level for each river node
RB(NRIV)	1-80	20F 4.0	Bottom elevation of the streambed layer for each river node
RC(NRIV)	1-80	10F 8.0	Leakage factor for each river node

The RH, RB, RC arrays each preceded by a multiplication factor card.

<u>VARIABLE</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>DEFINITION</u>
FAC	1-10	G 10.0	The multiplication factor for the following set of data cards
HMAX	1-10	F 10.5	Maximum iteration parameter, of the sequence of parameters computed by the equations in the problem are all close to 1.0 and if this results in slow convergence or even divergence, bypass the computations in the model and insert HMAX=0.99863 may give a satisfactory rate of convergence.

Group VI: Parameters that Change with the Pumping Period

The program has two options for the simulation period:

1. To simulate a given number of time steps, set TMAX to a value larger than the expected simulation period. The program will use NUMT, CDLT, and DELT as coded. If NUMT is greater than 50 change the dimension of ITTO in subroutine STEP to the appropriate size.
2. To simulate a given pumping period, set NUMT larger than the number required for the simulation period (for example, 50). The program will compute the exact DELT (which will be \leq DELT coded) and NUMT to arrive exactly at TMAX on the last time step.

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1	1-10	G10.0	<u>KP</u>	Number of the pumping period
	11-20	G10.0	<u>KPM1</u>	Number of the previous pumping period
				Note: KPM1 is currently not used.
	21-30	G10.0	<u>NWEL</u>	Number of wells for this pumping period
	31-40	G10.0	<u>TMAX</u>	Number of days in this pumping period
	41-50	G10.0	<u>NUMT</u>	Number of time steps
	51-60	G10.0	<u>CDLT</u>	Multiplying factor for DELT
				Note: 1.5 is commonly used.
	61-70	G10.0	<u>DELT</u>	Initial time step in hours

The following data set depends on the variable IALT. DATA SET A is the case of original program, DATA SET B is the case of program modified for Cache Valley Model.

If NWEL = 0 the following set of cards is omitted.

<u>DATA SET A</u> (NWEL cards)				
<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>	
1-10	G10.0	K	Layer in which well is located	
11-20	G10.0	I	Row location of well	
21-30	G10.0	J	Column location of well	
31-40	G10.0	WELL(I,J,K)	Pumping rate (L^3/t), negative for a pumping well.	

DATA SET B

<u>CARD</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
2	1-10	G10.0	AWELL	Average discharge for "small" wells
	11-20	G10.0	NBWEL	Number of "big" wells
	21-30	G10.0	BWC	Discharge multiplication factor for big well for each pumping period
	31-40	G10.0	FBWELL	Time length factor of pumping for big well
	41-50	G10.0	NCF	Number of recharge constant flux node
	51-60	G10.0	CFC	Multiplication factor for constant flux

(NBWEL Cards)

<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1-10	G10.0	K	Layer in which well located
11-20	G10.0	I	Row location of well
21-30	G10.0	J	Column location of well
31-40	G10.0	BWELL(I,J,K)	Pumping rate (L^3/t), negative for "big" well, negative value

(NCF Cards)

1-10	G10.0	K	Layer in which well located
11-20	G10.0	I	Row location of well
21-30	G10.0	J	Column location of well
31-40	G10.0	RCF(I,J,K)	Recharge constant flux rate (L^3/t), positive value

<u>DATA SET</u>	<u>COLUMNS</u>	<u>FORMAT</u>	<u>VARIABLE</u>	<u>DEFINITION</u>
1	1-80	20G 4.0	NWELL(I,J,K)	Total number of "small" wells, within the cell, for each node

For each additional pumping period, another set of group VI cards is required (that is, NPER sets of group VI cards are required).

Appendix C

Model Input Data of Transient-state Calibration of

March 1969

CACHE VALLEY GROUND-WATER MODELING

DRAW HEAD PASS	23	39	DATE RECH	50	30	31	75	MILES
	5280	5280	.01	3-3-405-08	1412	10	10	

1.	1.	1.	1.	1.	1.	1.	1.	
2.	0.	0.	0.	0.	0.	0.	0.	
3.	0.	0.	0.	0.	0.	0.	0.	
4.	0.	0.	0.	0.	0.	0.	0.	
5.	0.	0.	0.	0.	0.	0.	0.	
6.	0.	0.	0.	0.	0.	0.	0.	
7.	0.	0.	0.	0.	0.	0.	0.	
8.	0.	0.	0.	0.	0.	0.	0.	
9.	0.	0.	0.	0.	0.	0.	0.	
10.	0.	0.	0.	0.	0.	0.	0.	
11.	0.	0.	0.	0.	0.	0.	0.	
12.	0.	0.	0.	0.	0.	0.	0.	
13.	0.	0.	0.	0.	0.	0.	0.	
14.	0.	0.	0.	0.	0.	0.	0.	
15.	0.	0.	0.	0.	0.	0.	0.	
16.	0.	0.	0.	0.	0.	0.	0.	
17.	0.	0.	0.	0.	0.	0.	0.	
18.	0.	0.	0.	0.	0.	0.	0.	
19.	0.	0.	0.	0.	0.	0.	0.	
20.	0.	0.	0.	0.	0.	0.	0.	
21.	0.	0.	0.	0.	0.	0.	0.	
22.	0.	0.	0.	0.	0.	0.	0.	
23.	0.	0.	0.	0.	0.	0.	0.	
24.	0.	0.	0.	0.	0.	0.	0.	
25.	0.	0.	0.	0.	0.	0.	0.	
26.	0.	0.	0.	0.	0.	0.	0.	
27.	0.	0.	0.	0.	0.	0.	0.	
28.	0.	0.	0.	0.	0.	0.	0.	
29.	0.	0.	0.	0.	0.	0.	0.	
30.	0.	0.	0.	0.	0.	0.	0.	
31.	0.	0.	0.	0.	0.	0.	0.	
32.	0.	0.	0.	0.	0.	0.	0.	
33.	0.	0.	0.	0.	0.	0.	0.	
34.	0.	0.	0.	0.	0.	0.	0.	
35.	0.	0.	0.	0.	0.	0.	0.	
36.	0.	0.	0.	0.	0.	0.	0.	
37.	0.	0.	0.	0.	0.	0.	0.	
38.	0.	0.	0.	0.	0.	0.	0.	
39.	0.	0.	0.	0.	0.	0.	0.	
40.	0.	0.	0.	0.	0.	0.	0.	
41.	0.	0.	0.	0.	0.	0.	0.	
42.	0.	0.	0.	0.	0.	0.	0.	
43.	0.	0.	0.	0.	0.	0.	0.	
44.	0.	0.	0.	0.	0.	0.	0.	
45.	0.	0.	0.	0.	0.	0.	0.	
46.	0.	0.	0.	0.	0.	0.	0.	
47.	0.	0.	0.	0.	0.	0.	0.	
48.	0.	0.	0.	0.	0.	0.	0.	
49.	0.	0.	0.	0.	0.	0.	0.	
50.	0.	0.	0.	0.	0.	0.	0.	
51.	0.	0.	0.	0.	0.	0.	0.	
52.	0.	0.	0.	0.	0.	0.	0.	
53.	0.	0.	0.	0.	0.	0.	0.	
54.	0.	0.	0.	0.	0.	0.	0.	
55.	0.	0.	0.	0.	0.	0.	0.	
56.	0.	0.	0.	0.	0.	0.	0.	
57.	0.	0.	0.	0.	0.	0.	0.	
58.	0.	0.	0.	0.	0.	0.	0.	
59.	0.	0.	0.	0.	0.	0.	0.	
60.	0.	0.	0.	0.	0.	0.	0.	
61.	0.	0.	0.	0.	0.	0.	0.	
62.	0.	0.	0.	0.	0.	0.	0.	
63.	0.	0.	0.	0.	0.	0.	0.	
64.	0.	0.	0.	0.	0.	0.	0.	
65.	0.	0.	0.	0.	0.	0.	0.	
66.	0.	0.	0.	0.	0.	0.	0.	
67.	0.	0.	0.	0.	0.	0.	0.	
68.	0.	0.	0.	0.	0.	0.	0.	
69.	0.	0.	0.	0.	0.	0.	0.	
70.	0.	0.	0.	0.	0.	0.	0.	
71.	0.	0.	0.	0.	0.	0.	0.	
72.	0.	0.	0.	0.	0.	0.	0.	
73.	0.	0.	0.	0.	0.	0.	0.	
74.	0.	0.	0.	0.	0.	0.	0.	
75.	0.	0.	0.	0.	0.	0.	0.	
76.	0.	0.	0.	0.	0.	0.	0.	
77.	0.	0.	0.	0.	0.	0.	0.	
78.	0.	0.	0.	0.	0.	0.	0.	
79.	0.	0.	0.	0.	0.	0.	0.	
80.	0.	0.	0.	0.	0.	0.	0.	
81.	0.	0.	0.	0.	0.	0.	0.	
82.	0.	0.	0.	0.	0.	0.	0.	
83.	0.	0.	0.	0.	0.	0.	0.	
84.	0.	0.	0.	0.	0.	0.	0.	
85.	0.	0.	0.	0.	0.	0.	0.	
86.	0.	0.	0.	0.	0.	0.	0.	
87.	0.	0.	0.	0.	0.	0.	0.	
88.	0.	0.	0.	0.	0.	0.	0.	
89.	0.	0.	0.	0.	0.	0.	0.	
90.	0.	0.	0.	0.	0.	0.	0.	
91.	0.	0.	0.	0.	0.	0.	0.	
92.	0.	0.	0.	0.	0.	0.	0.	
93.	0.	0.	0.	0.	0.	0.	0.	
94.	0.	0.	0.	0.	0.	0.	0.	
95.	0.	0.	0.	0.	0.	0.	0.	
96.	0.	0.	0.	0.	0.	0.	0.	
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99.	0.	0.	0.	0.	0.	0.	0.	
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113.	0.	0.	4515.	4515.	4515.	4515.	0.	
114.	0.	0.	0.	4475.	4495.	0.	0.	
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132.	0.	0.	0.	0.	0.	0.	0.	
133.	0.	0.	0.	0.	0.	0.	0.	
134.	0.	0.	0.	0.	0.	0.	0.	
135.	0.	0.	0.	0.	0.	0.	0.	
136.	0.	0.	0.	0.	0.	0.	0.	
137.	0.	0.	0.	0.	0.	0.	0.	
138.	0.	0.	0.	0.	0.	0.	0.	
139.	0.	0.	0.	0.	0.	0.	0.	
140.	4440.	4430.	4424.	4424.	4420.	4407.	4470.	0.
141.	0.	0.	0.	0.	0.	0.	0.	0.
142.	0.	0.	0.	0.	0.	0.	0.	0.
143.	0.	0.	0.	0.	0.	0.	0.	0.
144.	4428.	4425.	4430.	4440.	4435.	4435.	4435.	4430.
145.	4428.	4421.	4416.	4413.	4420.	4407.	4480.	0.
146.	0.	0.	0.	0.	4480.	4512.	0.	0.
147.	0.	0.	0.	0.	0.	0.	0.	0.
148.	0.	0.	4460.	4443.	4425.	4417.	4415.	4410.
149.	4410.	4410.	4410.	4410.	4410.	4410.	4407.	4407.
150.	4413.	4407.	4407.	4407.	4407.	4407.	4403.	0.
151.	0.	0.	0.	4453.	4470.	4490.	0.	0.
152.	0.	0.	0.	0.	0.	0.	0.	0.
153.	0.	0.	4470.	4440.	4420.	4419.	4415.	4410.
154.	4410.	4410.	4410.	4410.	4410.	4410.	4407.	4407.
155.	4407.	4405.	4415.	4407.	4410.	4413.	4418.	4430.
156.	4400.	4403.	4403.	4442.	4478.	4460.	0.	0.
157.	0.	0.	0.	0.	0.	4545.	4535.	4500.
158.	4480.	4472.	4429.	4421.	4420.	4418.	4415.	4410.
159.	4410.	4412.	4410.	4410.	4410.	4410.	4410.	4410.
160.	4415.	4420.	4407.	4407.	4417.	4410.	4410.	4415.
161.	4420.	4440.	4426.	4426.	4420.	4440.	0.	0.
162.	0.	0.	0.	0.	0.	4520.	4520.	4470.
163.	4427.	4430.	4430.	4435.	4425.	4419.	4415.	4415.
164.	4415.	4414.	4413.	4410.	4413.	4412.	4410.	4410.
165.	4417.	4415.	4410.	4421.	4421.	4413.	4412.	4410.
166.	4411.	4445.	4430.	4445.	4435.	4465.	0.	0.
167.	0.	0.	0.	0.	0.	4580.	4523.	4480.
168.	4448.	4439.	4440.	4440.	4434.	4430.	4421.	4423.
169.	4423.	4418.	4415.	4410.	4417.	4413.	4413.	4414.
170.	4419.	4410.	4415.	4415.	4435.	4423.	4414.	4413.
171.	4412.	4426.	4430.	4465.	4475.	4465.	0.	0.
172.	0.	0.	0.	0.	0.	4520.	4520.	4700.
173.	4429.	4457.	4452.	4445.	4439.	4421.	4421.	4427.
174.	4427.	4423.	4418.	4423.	4423.	4421.	4421.	4417.
175.	4421.	4420.	4421.	4430.	4433.	4430.	4420.	4418.
176.	4411.	4421.	4432.	4463.	4466.	4475.	0.	0.
177.	0.	0.	0.	0.	0.	4610.	4540.	4510.
178.	4469.	4465.	4454.	4442.	4429.	4432.	4431.	4431.
179.	4431.	4426.	4426.	4429.	4429.	4427.	4427.	4424.
180.	4424.	4425.	4426.	4415.	4424.	4420.	4430.	4430.
181.	4425.	4420.	4420.	4464.	4491.	4504.	0.	0.
182.	0.	0.	0.	0.	0.	4520.	4545.	4720.
183.	4492.	4484.	4480.	4470.	4452.	4440.	4417.	4435.
184.	4430.	4425.	4425.	4433.	4433.	4429.	4427.	4427.
185.	4425.	4430.	4427.	4425.	4430.	4410.	4435.	4425.
186.	4425.	4425.	4425.	4467.	4470.	4502.	0.	0.
187.	0.	0.	0.	0.	0.	4590.	0.	4525.
188.	4492.	4482.	4480.	4480.	4463.	4444.	4441.	4440.
189.	4434.	4432.	4430.	4430.	4436.	4432.	4430.	4429.
190.	4427.	4433.	4430.	4430.	4436.	4435.	4420.	4430.
191.	4430.	4435.	4435.	4468.	4495.	4502.	0.	0.
192.	0.	0.	0.	0.	0.	4644.	4650.	0.
193.	4507.	4495.	4490.	4486.	4480.	4486.	4460.	4450.
194.	4452.	4448.	4440.	4440.	4440.	4440.	4410.	4437.
195.	4437.	4437.	4432.	4438.	4450.	4420.	4440.	4445.
196.	4420.	4440.	4420.	4475.	4497.	4504.	0.	0.
197.	0.	4790.	4785.	4780.	4775.	4700.	0.	0.
198.	4512.	4501.	4500.	4502.	4490.	4480.	4479.	4463.
199.	4467.	4462.	4454.	4450.	4451.	4450.	4450.	4450.
200.	4444.	4441.	4439.	4443.	4464.	4470.	4460.	4460.
201.	4440.	4442.	4432.	4438.	4450.	4470.	4460.	4460.
202.	0.	4810.	4779.	4795.	4780.	4725.	4700.	0.
203.	0.	4515.	4515.	4505.	4495.	4487.	4478.	4474.
204.	4480.	4478.	4480.	4475.	4475.	4470.	4476.	4469.
205.	4433.	4445.	4445.	4460.	4460.	4470.	4482.	4480.
206.	4465.	4462.	4460.	4465.	4476.	4500.	0.	0.
207.	0.	4825.	4815.	4800.	4782.	4763.	4725.	0.
208.	0.	4515.	4515.	4510.	4499.	4498.	4489.	4487.
209.	4490.	4500.	4510.	4500.	4493.	4490.	4486.	4486.
210.	4467.	4451.	4455.	4476.	4490.	0.	0.	0.
211.	4485.	4472.	4460.	4440.	4460.	4499.	0.	0.
212.	0.	0.	0.	0.	4770.	4772.	4752.	0.
213.	0.	0.	4530.	4520.	4520.	4515.	4510.	4502.
214.	4505.	4510.	4500.	4515.	4519.	4502.	4502.	4492.
215.	4488.	4473.	4475.	4470.	0.	0.	0.	0.
216.	4519.	4495.	4485.	4455.	4450.	4460.	0.	0.
217.	0.	0.	0.	0.	0.	0.	0.	0.
218.	0.	0.	0.	0.	0.	0.	0.	0.
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220.	4515.	4495.	0.	0.	4475.	4480.	4475.	0.
221.	0.	0.	0.	0.	0.	0.	0.	0.
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224.	0.	0.	0.	0.	0.	0.	0.	0.

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483.	4457.	4333.	4430.	4430.	4435.	4435.	4444.
484.	4445.	4341.	4450.	4471.	4501.	4504.	4570.
485.	0.	0.	0.	0.	0.	4464.	4466.
486.	4522.	4506.	4500.	4476.	4484.	4466.	4460.
487.	4452.	4448.	4440.	4440.	4440.	4440.	4437.
488.	4437.	4431.	4439.	4450.	4450.	4445.	4445.
489.	4440.	4445.	4420.	4477.	4501.	4505.	0.
490.	0.	4800.	4800.	4810.	4864.	4864.	4574.
491.	4510.	4311.	4520.	4512.	4495.	4469.	4443.
492.	4467.	4463.	4454.	4450.	4451.	4450.	4450.
493.	4444.	4441.	4439.	4443.	4464.	4463.	4480.
494.	4450.	4442.	4430.	4483.	4503.	4505.	0.
495.	0.	4520.	4805.	4805.	4770.	4773.	4690.
496.	4540.	4540.	4520.	4520.	4505.	4487.	4470.
497.	4482.	4473.	4501.	4485.	4485.	4480.	4460.
498.	4453.	4442.	4445.	4460.	4495.	4520.	4510.
499.	4445.	4442.	4430.	4488.	4504.	4501.	0.
500.	0.	4840.	4830.	4810.	4793.	4791.	4930.
501.	4602.	4579.	4520.	4523.	4510.	4490.	4480.
502.	4473.	4507.	4520.	4511.	4503.	4479.	4473.
503.	4467.	4451.	4435.	4476.	4520.	4570.	4545.
504.	4484.	4475.	4470.	4435.	4505.	4495.	0.
505.	0.	0.	0.	0.	4810.	4804.	4785.
506.	4597.	4579.	4553.	4540.	4560.	4550.	4520.
507.	4533.	4524.	4520.	4548.	4541.	4534.	4525.
508.	4488.	4473.	4475.	4490.	4540.	4610.	4620.
509.	4523.	4495.	4499.	4445.	4500.	4483.	0.
510.	0.	0.	0.	0.	0.	4850.	4817.
511.	4460.	4425.	4405.	4443.	4473.	4470.	4590.
512.	4543.	4509.	4533.	4535.	4620.	4600.	4521.
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516.	4800.	4820.	4710.	4820.	4890.	4860.	4800.
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521.	0.	0.	0.	0.	0.	0.	0.
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532.	0.	0.	0.	0.	0.	0.	0.
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585.	0.	0.	0.	0.	0.	0.	0.
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595.	0.	0.	0.	0.	0.	0.	0.
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605.	0.	0.	0.	0.	0.	0.	0.
606.	0.	0.	0.	0.	0.	0.	0.
607.	0.	0.	0.	0.	0.	0.	0.
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610.	0.	0.	0.	0.	0.	0.	0.
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612.	0.	0.	0.	0.	0.	0.	0.
613.	0.	0.	0.	0.	0.	0.	0.
614.	0.	0.	0.	0.	0.	0.	0.
615.	0.	0.	0.	0.	0.	0.	0.
616.	0.	0.	0.	0.	0.	0.	0.
617.	0.	0.	0.	0.	0.	0.	0.
618.	0.	0.	0.	0.	0.	0.	0.
619.	0.	0.	0.	0.	0.	0.	0.
620.	0.	0.	0.	0.	0.	0.	0.
621.	0.	0.	0.	0.	0.	0.	0.
622.	0.	0.	0.	0.	0.	0.	0.
623.	0.	0.	0.	0.	0.	0.	0.
624.	0.	0.	0.	0.	0.	0.	0.
625.	0.	0.	0.	0.	0.	0.	0.
626.	0.	0.	0.	0.	0.	0.	0.
627.	0.	0.	0.	0.	0.	0.	0.
628.	0.	0.	0.	0.	0.	0.	0.
629.	0.	0.	0.	0.	0.	0.	0.
630.	0.	0.	0.	0.	0.	0.	0.
631.	0.	0.	0.	0.	0.	0.	0.
632.	0.	0.	0.	0.	0.	0.	0.
633.	0.	0.	0.	0.	0.	0.	0.
634.	0.	0.	0.	0.	0.	0.	0.
635.	0.	0.	0.	0.	0.	0.	0.
636.	0.	0.	0.	0.	0.	0.	0.
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640.	0.	0.	0.	0.	0.	0.	0.
641.	0.	0.	0.	0.	0.	0.	0.
642.	0.	0.	0.	0.	0.	0.	0.
643.	0.	0.	0.	0.	0.	0.	0.
644.	0.	0.	0.	0.	0.	0.	0.
645.	0.	0.	0.	0.	0.	0.	0.
646.	0.	0.	0.	0.	0.	0.	0.
647.	0.	0.	0.	0.	0.	0.	0.
648.	0.	0.	0.	0.	0.	0.	0.
649.	0.	0.	0.	0.	0.	0.	0.
650.	0.	0.	0.	0.	0.	0.	0.
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670.	0.	0.	0.	0.	0.	0.	0.
671.	0.	0.	0.	0.	0.	0.	0.
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673.	0.	0.	0.	0.	0.	0.	0.
674.	0.	0.	0.	0.	0.	0.	0.
675.	0.	0.	0.	0.	0.	0.	0.
676.	0.	0.	0.	0.	0.	0.	0.
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תְּמִימָנָה וְעַמְּדָה
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Appendix D

Model Output of Transient-state Calibration of

March 1969



CACER VALLEY ENGINEERING APPLICANT

DIRECTIONAL TRANSMISSIBILITY MULTIPLICATION FACTORS FOR LAYER 1

X * 1,000,000
Y * 1,000,000
Z * 1,000,000

TRANSMISSIVITY = .0000000 FOR LAYER 2
 DIRECTIONAL TRANSMISSIVITY MULTIPLICATION FACTORS FOR LAYER 2
 X = 1.000000
 Y = 1.000000
 Z = 1.000000

RECHARGE RATE = .0000000 FOR LAYER 1

GRID SPACING IN PROTOTYPE IN X DIRECTION

5280.	3760.	3760.	3760.	3760.	3760.	3760.	3760.	3760.	3760.	3760.	3760.	3760.	3760.
2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.
3760.	3760.	3760.	3760.	3760.	3280.	3280.	3280.	3280.	3280.	3280.	3280.	3280.	3280.
10560.	7720.	7720.											

GRID SPACING IN PROTOTYPE IN Y DIRECTION

5280.	5280.	3760.	3760.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.
2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.	2640.

DELT = 1.000000

.5000-008

1.	10.	5.	10.	10.	2.	1.	1.	1.	1.	50.	50.	1.	20.	10.	7.	1.	1.	1.
1.																		

1.000

4440, 4447, 4533, 4500, 4475, 4450, 4423, 4430, 4443, 4433, 4435, 4446, 4430, 4440, 4470, 4460, 4470, 4470, 4470, 4470, 4470, 4470, 4470,

4460,

1.000

4420, 4408, 4409, 4407, 4407, 4423, 4420, 4418, 4416, 4413, 4412, 4410, 4408, 4408, 4407, 4407, 4423, 4413, 4410, 4470,

4460, 4430, 4418, 4415, 4414, 4407, 4450, 4440, 4427, 4423, 4415, 4408, 4500, 4423, 4420, 4409, 4421, 4523, 4411, 4427,

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1.000

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4433, 4433, 4413, 4410, 4409, 4402, 4445, 4433, 4422, 4418, 4410, 4403, 4500, 4430, 4413, 4404, 4416, 4520, 4436, 4424,

4417, 4403, 4406, 4407, 4415, 4533, 4428, 4420, 4409, 4410, 4414, 4500, 4433, 4423, 4430, 4409, 4480, 4445, 4451, 4700,

4700, 4480, 4463, 4480, 4463, 4470, 4480, 4470, 4490, 4481, 4481, 4523, 4510, 4510, 4520, 4460,

1.000

.5000-009 *

50.	100.	100.	100.	100.	300.	200.	300.	300.	300.	300.	300.	100.	100.	100.
100.	100.	300.	200.	100.	50.	100.	100.	200.	100.	50.	100.	100.	100.	100.
100.	200.	300.	100.	50.	100.	200.	10.	50.	50.	100.	100.	200.	100.	100.
50.	100.	100.	50.	100.	300.	100.	50.	10.	10.	100.	100.	100.	50.	50.
100.	100.	100.	50.	50.	100.	100.	100.	100.	100.	100.	100.	100.	100.	100.
100.	50.	100.	100.	100.	20.									

SOLUTION BY THE STRONGLY IMPLICIT PROCEDURE

HMAX = .99863

5 ITERATION PARAMETERS: .0000000 ,8074111+000 -7429845+000 ,9929790+000 ,9986300+000
 PUMPING PERIOD NO.: 11 365.00 DAYS

NUMBER OF TIME STEPS= 1

DELT IN HOURS = 8760.000

MULTIPLIER FOR DELT = 1.000
 485 WELLS

R	I	J	PUMPING RATE
1	12	12	-61922
1	13	12	-1.43944
1	14	15	-80774
1	14	19	-67420
1	14	20	-2.88492
1	14	39	-4.15436
1	14	31	-1.64000
1	15	8	-2.22068
1	15	12	-9.13488
1	15	26	-9.29000
1	15	24	-1.12700
1	15	34	-1.47004
1	15	31	-7.90160
1	15	32	-1.49580
1	16	18	-2.13388
1	16	30	-3.76372
1	17	7	-4.97748
1	17	17	-4.48776
1	17	21	-4.07768
1	17	29	-1.34076
1	17	30	-6.95808
1	17	31	-7.17140
1	18	8	-8.1972
1	18	9	-7.78294
1	18	18	-2.12188
1	18	15	-10.71348
1	18	21	-7.33340
1	18	28	-1.62944
1	18	27	-5.17392
1	18	30	-2.45916

1.13900
1.127912
1.240584
1.529890
1.594372
1.598888
1.544524
1.548853
1.515238
1.552590
1.522990
1.438000
1.431000
1.414000
1.345000
1.207000
1.069000
1.069000
1.067000
1.138000
1.227000
1.207000
1.174000
1.215000
1.100000
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1.114000
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1.128000
1.138000
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1.290000
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.135000
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			1	21	38	.27600
			1	22	39	.96600
-8000-002	39	.9200	.4850-001	105	1.000	
8	9	2	45.77857			
8	7	2	-44.44767			
8	19	2	-3.113431			
15	15	1	1.194072			
26	26	1	7164294			
14	7	1	1246998			
16	5	1	1423444			
13	24	1	1246998			
15	24	1	7607473-001			
17	23	1	1250709			
17	23	1	2089943-001			
17	24	1	1298361-001			
13	24	1	1201574-001			
19	26	1	1786300-001			
16	17	2	15849619-002			

TIME STEP NUMBER = 1

SIZE OF TIME STEP IN SECONDS= 31536000.00

TOTAL SIMULATION TIME IN SECONDS= 31536000.00
 MINUTES= 525000.00
 HOURS= 8750.00
 DAYS= 345.00
 YEARS= 1.00

DURATION OF CURRENT PUMPING PERIOD IN DAYS= 345.00
 YEARS= 1.00

CUMULATIVE-MASS BALANCE:	L143	RATES FOR THIS TIME STEP:	L143/T
<u>SOURCES:</u>			
STORAGE =	144695756.00	STORAGE =	4.5803
RECHARGE =	3584054064.00	RECHARGE =	113.6427
CONSTANT FLUX =	3622918144.00	CONSTANT FLUX =	114.8820
CONSTANT HEAD =	50029967.50	PUMPING =	-30.5793
LEAKAGE =	.00	RIVER LEAKAGE =	-34.6318
TOTAL SOURCES =	7401699964.00	CONSTANT HEAD =	
<u>DISCHARGES:</u>			
RIVER LEAKAGE =	1091633440.00	IN =	1.5664
CONSTANT HEAD =	1392799204.00	OUT =	-44.1484
QUANTITY PUMPED =	264357232.00	LEAKAGE =	
SPRING LEAKAGE =	928493880.00	FROM PREVIOUS PUMPING PERIOD =	-95.0131
EVAPOTRANSPIRATION =	3014446432.00	TOTAL =	-29.1487
TOTAL DISCHARGE =	7402132352.00	EVAPOTRANSPIRATION =	-95.2046
DISCHARGE-SOURCES =	432448.00	SUM OF RATES =	-9.9137
PER CENT DIFFERENCE =	.01		

RATES OF COMPONENTS FOR EACH LAYER IN CFS
 LAYER 1 CHOUT=.00000 DHIN=.00000 ORE=.00000 FWP=-30.524 CFUX=.114.89 STORE=.80403-002
 RIVER=.00000 SPRING=.00000 ET=.00000 PWP=-.56000-001 CFUX=.00000 STORE=4.5963
 LAYER 2 CHOUT=-44.165 DHIN=1.2844 ORE=113.65 FWP=-.56000-001 CFUX=.00000 STORE=4.5963
 RIVER=-34.622 SPRING=-1.449 SPRING=ET=.95.905

FLOW RATES TO CONSTANT HEAD NODES:

K	I	J	RATE (L143/T)	K	I	J	RATE (L143/T)	K	I	J	RATE (L143/T)
1	3	36	-2.150145	2	4	36	-1.8430391	3	23		-1.3595612
3	35	36	-1.2541263	3	5	37	-1.2119796	5	28		-1.1252281
3	29		-1.4548391	3	6	33	-3.1287642	6	24		-3.631932
3	13		-1.1380224	3	7	34	-1.728724	7	25		-1.7184831
2	36		-1.163009	2	7	38	-5.089421	8	33		-1.3705114
8	37		-5.0523660	9	9	35	-1.5773978	10	38		-1.4284152
11	33		-8996071	12	12	35	-1.5111712	13	14		-1.3260281-001
14	6		-1.227105	14	13	35	-1.430117	15	5		-1.237763
15	6		-1.9006292	15	15	35	-1.6226834	16	33		-2.152291
17	34		-3.963056	18	18	37	-6.107289	19	38		-2.878010

FLOW TO TOP LAYER = 84.07143 FLOW TO BOTTOM LAYER = 83.57948 POSITIVE UPWARD

SPRING 5 11 -.6532651-001
 SPRING 6 10 -.6571783-001
 SPRING 7 6 -.2534016+000
 SPRING 7 6 -.2513855-002
 SPRING 7 9 -.2454082-001
 SPRING 8 8 -.2129653+000
 SPRING 11 14 -.6698786+000
 SPRING 12 14 -.6297910+000
 SPRING 14 14 -.1328250+000
 SPRING 14 21 -.2024142+000

SPRIND 14 23 -.1015443+000
SPRIND 14 30 -.7707585+000
SPRIND 14 31 -.7870594+000
SPRIND 15 27 -.1396471+000
SPRIND 15 30 -.1315146+000
SPRIND 15 31 -.2433577+000
SPRIND 16 3 .2334811-001
SPRIND 16 29 -.6160511-001
SPRIND 16 14 .1112279+000
SPRIND 16 15 .1038991+000
SPRIND 17 37 -.7380260+000

TOTAL SPRIND LIAGE RCHD 2ND LAYER -.5482200+001

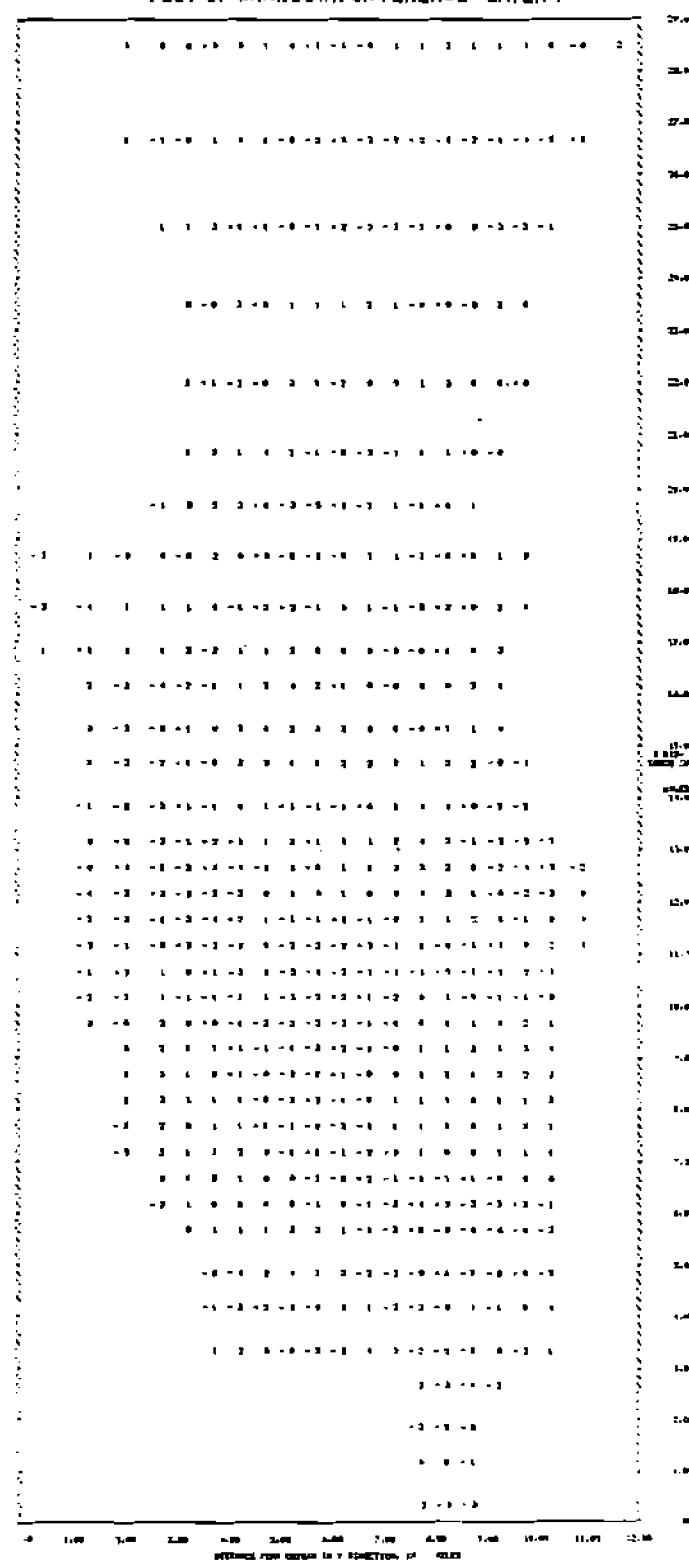
MAXIMUM HEAD CHANGE FOR EACH ITERATION:

45.7786	44.4477	3.1136	1.1941	.7184	.2258	.1423	.1247	.0781	.1221
.0207	.0160	.0120	.0179	.0058					

TIME STEP : 1

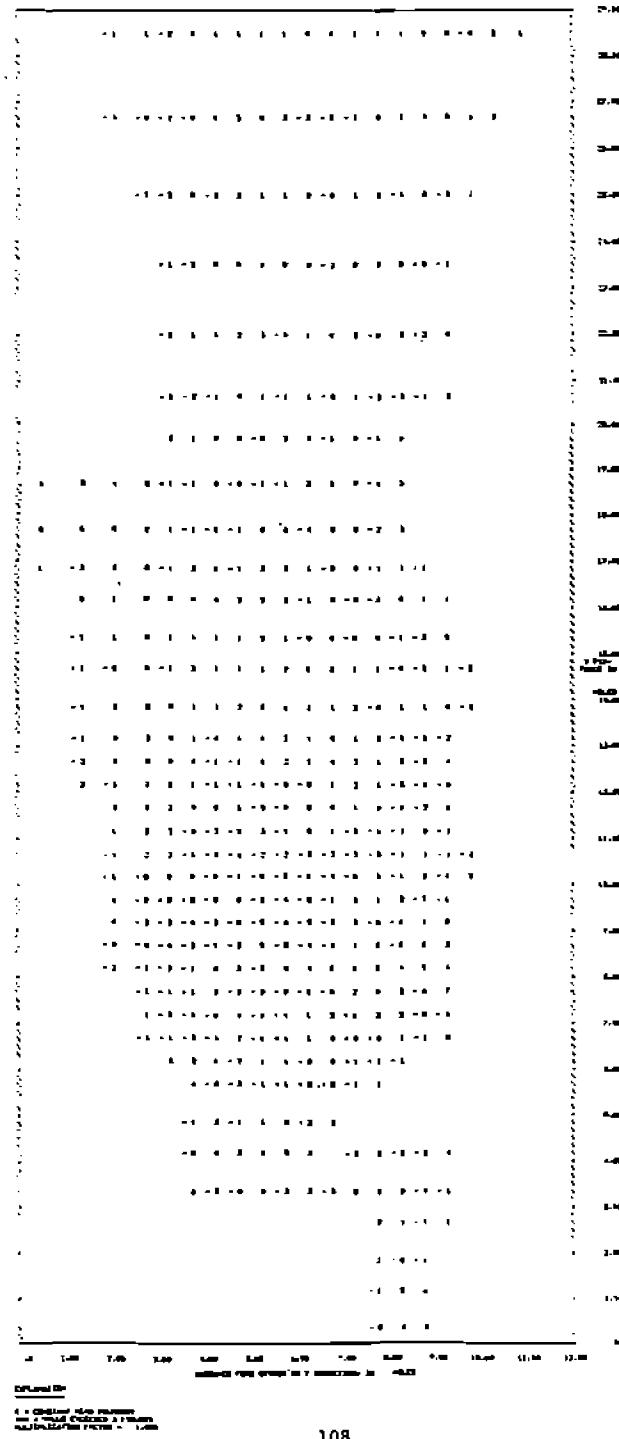
ITERATIONS: 14

PLOT OF DRAWDOWN DIFFERENCE LAYER 1



EXPLANATION:
1000' = 1000 FT. SURFACE
1000' = 1000 FT. DEPTH
MULTIPLICATION FACTOR = 1.000

PLOT OF DRAWDOWN DIFFERENCE LAYER 2



HEAD MATRIX, LATER 1

1	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
2	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	1442.24	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	1449.15	1441.04	
3	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	1503.40	4493.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	4454.23	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	1435.59	1433.89	
4	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	4482.60	4487.71	
	1463.23	4460.16	4454.24	4452.65	4448.80	4445.04	4441.33	4437.76	4435.29	4433.00	4430.70	4429.50	1431.76	4434.61	4439.92		
	1457.19	.00	.00	.00	.00	.00	4488.16	4501.72	.00	.00	.00	.00	.00	.00	.00	.00	
5	.00	.00	.00	.00	.00	.00	.00	.00	.00	4486.24	4479.92	1478.35	4473.29	4468.37	4463.52		
	1458.20	4454.34	4451.19	4448.04	4443.68	4440.04	4436.82	4434.29	4432.73	4430.72	4433.12	4433.17	4434.42	4436.09	1440.33		
	1457.72	4478.00	.00	.00	4478.48	4483.32	4496.31	.00	.00	.00	.00	.00	.00	.00	.00	.00	
6	.00	.00	.00	.00	.00	.00	.00	.00	.00	4485.76	4478.52	4474.55	4473.24	1467.53	4465.27	4462.27	
	1458.39	4454.89	4451.78	4448.97	4444.72	4441.13	4438.26	4435.72	4434.73	1476.24	4436.27	1434.13	1417.31	4437.81	4440.72		
	1450.23	4465.14	4473.13	1471.74	4470.93	4471.30	4478.17	4489.70	.00	.00	.00	.00	.00	.00	.00	.00	
7	.00	.00	.00	.00	.00	4574.13	4534.09	4500.73	4483.35	4476.75	4473.77	4470.85	1448.47	4455.70	1463.89		
	4459.40	4454.35	4453.54	4450.77	4447.79	4425.79	4423.29	4421.70	4419.76	4410.92	4410.12	4409.53	1411.34	4411.51	1412.23		
	4446.35	4454.25	4460.34	4462.71	4462.71	4464.30	4464.20	4472.69	4483.10	.00	.00	.00	.00	.00	.00	.00	

HEAD MATRIX, LAYER 3

1	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
2	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4421.14	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
3	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4468.43	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4429.71	4427.60	4425.20	4431.33	4438.71	4434.79	4436.48	4431.74	4429.72	4427.72	4421.10	4415.01	4414.11	4417.30	4407.90	
4476.16	.00	.00	.00	.00	.00	.00	.00	4484.12	4512.79	.00	.00	.00	.00	.00	
5	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4413.33	4411.72	4410.42	4408.48	4408.44	4407.60	4407.40	4407.00	4408.48	4411.20	4407.00	4407.00	4407.00	4407.00	4407.00	
4452.70	.00	.00	.00	.00	.00	4425.57	4470.38	4488.57	.00	.00	.00	.00	.00	.00	
6	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4412.97	4411.85	4409.79	4409.39	4408.35	4407.34	4407.18	4407.00	4407.00	4411.33	4414.12	4407.00	4411.15	4412.10	4415.79	
4419.49	4426.53	4420.93	4424.92	4425.00	4426.81	4426.45	4422.16	.00	.00	.00	.00	.00	.00	.00	
7	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4414.01	4412.82	4411.80	4410.87	4410.10	4409.53	4409.20	4409.00	4410.15	4413.54	4416.39	4415.67	4407.00	4415.03	4411.11	
4411.16	4414.10	4422.14	4419.38	4412.70	4425.00	4429.11	4449.00	.00	.00	.00	.00	.00	.00	.00	
8	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4417.92	4415.13	4414.30	4413.41	4413.11	4412.63	4412.50	4411.50	4410.41	4413.71	4414.16	4408.70	4417.00	4419.75	4412.82	
4411.68	4409.82	4412.13	4444.32	4430.00	4447.19	4435.00	4443.79	.00	.00	.00	.00	.00	.00	.00	
9	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4423.36	4422.58	4418.79	4416.18	4415.04	4416.13	4414.19	4414.02	4413.38	4417.43	4409.04	4414.20	4423.37	4426.95	4421.24	
4414.41	4412.74	4411.74	4432.67	4430.00	4429.75	4437.31	4463.67	.00	.00	.00	.00	.00	.00	.00	
10	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4431.75	4429.76	4423.27	4419.73	4419.48	4422.14	4421.53	4417.57	4415.91	4419.48	4419.19	4419.81	4425.47	4431.18	4429.94	
4421.13	4418.45	4416.45	4422.00	24	4430.00	4462.36	4479.52	4494.34	.00	.00	.00	.00	.00	.00	
11	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
4437.33	4434.52	4426.34	4427.32	4426.37	4428.23	4422.43	4423.43	4422.10	4422.03	4422.63	4424.52	4425.33	4413.09	4420.71	4414.34

12	.00	.00	.00	.00	.00	4617.79	4542.14	4577.68	4493.58	4484.33	4479.41	4469.42	4454.37	4435.61	4441.08	
	4440.10	4435.14	4436.39	4428.05	4432.63	4432.93	4429.14	4425.22	4425.70	4424.34	4420.33	4427.35	4425.67	4428.69	4414.29	
	4435.37	4420.73	4433.70	4429.09	4421.00	4426.52	4424.05	4501.99	.00							
13	.00	.00	.00	.00	.00	4574.99	.00	4623.46	4591.80	4491.56	4484.51	4477.77	4463.23	4441.52	4446.71	
	4413.45	4434.76	4435.77	4433.26	4436.31	4433.59	4430.97	4429.58	4428.51	4426.36	4430.81	4429.67	4439.93	4436.13	4435.99	
	4417.12	4431.41	4430.43	4432.00	4426.37	4468.06	4476.70	4501.85	.00							
14	.00	.00	.00	.00	.00	4664.00	4654.94	.00	4507.58	4475.77	4470.48	4486.71	4477.91	4459.75	4455.75	
	4448.08	4447.73	4448.39	4442.66	4440.06	4430.56	4438.26	4437.43	4436.43	4435.16	4434.02	4432.41	4438.26	4450.00	4441.76	
	4430.88	4444.93	4418.72	4437.99	4420.00	4473.64	4492.82	4502.93	.00							
15	.00	4798.12	4786.86	4782.08	4664.00	4664.00	4674.24	.00	4511.29	4506.31	4500.04	4678.26	4489.59	4477.30	4462.65	
	4463.37	4466.09	4461.87	4454.28	4450.69	4450.59	4449.05	4449.23	4449.26	4444.01	4440.31	4438.80	4445.17	4465.34	4471.05	
	4460.69	4460.72	4463.26	4462.04	4425.00	4477.74	4497.69	4502.76	.00							
16	.00	4809.41	4789.62	4795.38	4779.26	4750.48	4701.74	.00	.00	4515.82	4508.94	4503.36	4493.95	4482.55	4471.61	
	4474.22	4477.67	4479.03	4480.67	4476.32	4476.37	4472.93	4467.99	4460.89	4453.71	4445.01	4445.67	4459.67	4479.09	4487.85	
	4481.76	4479.64	4462.29	4460.14	4430.00	4465.72	4474.90	4499.16	.00							
17	.00	4820.17	4809.00	4801.28	4783.91	4767.14	4728.00	.00	.00	.00	4515.64	4510.40	4495.88	4485.42	4476.54	
	4482.73	4472.21	4477.05	4508.47	4479.92	4491.70	4491.08	4486.61	4481.13	4466.37	4431.11	4436.88	4475.34	4491.37	.00	
	.00	4483.75	4476.69	4460.25	4440.00	4481.29	4489.86	.00								
18	.00	.00	.00	.00	.00	4786.79	4773.05	4749.02	.00	.00	.00	4529.57	4528.67	4518.15	4511.40	4507.03
	4501.75	4506.22	4513.66	4520.79	4516.35	4509.37	4505.27	4501.47	4494.61	4487.67	4472.33	4474.82	4489.04	.00	.00	
	.00	4507.81	4474.96	4483.92	4456.65	4430.00	4466.00	.00								
19	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	4522.93	4527.31	.00	.00	.00	.00	.00	.00	4516.34	4477.43	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	4473.93	4474.30	4475.14	.00							
20	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
21	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
22	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
23	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	

RECHARGE RATE = .00000000 FOR LAYER 1
 PUMPING PERIOD NO. 21 345.00 DAYS

NUMBER OF TIME STEPS= 1

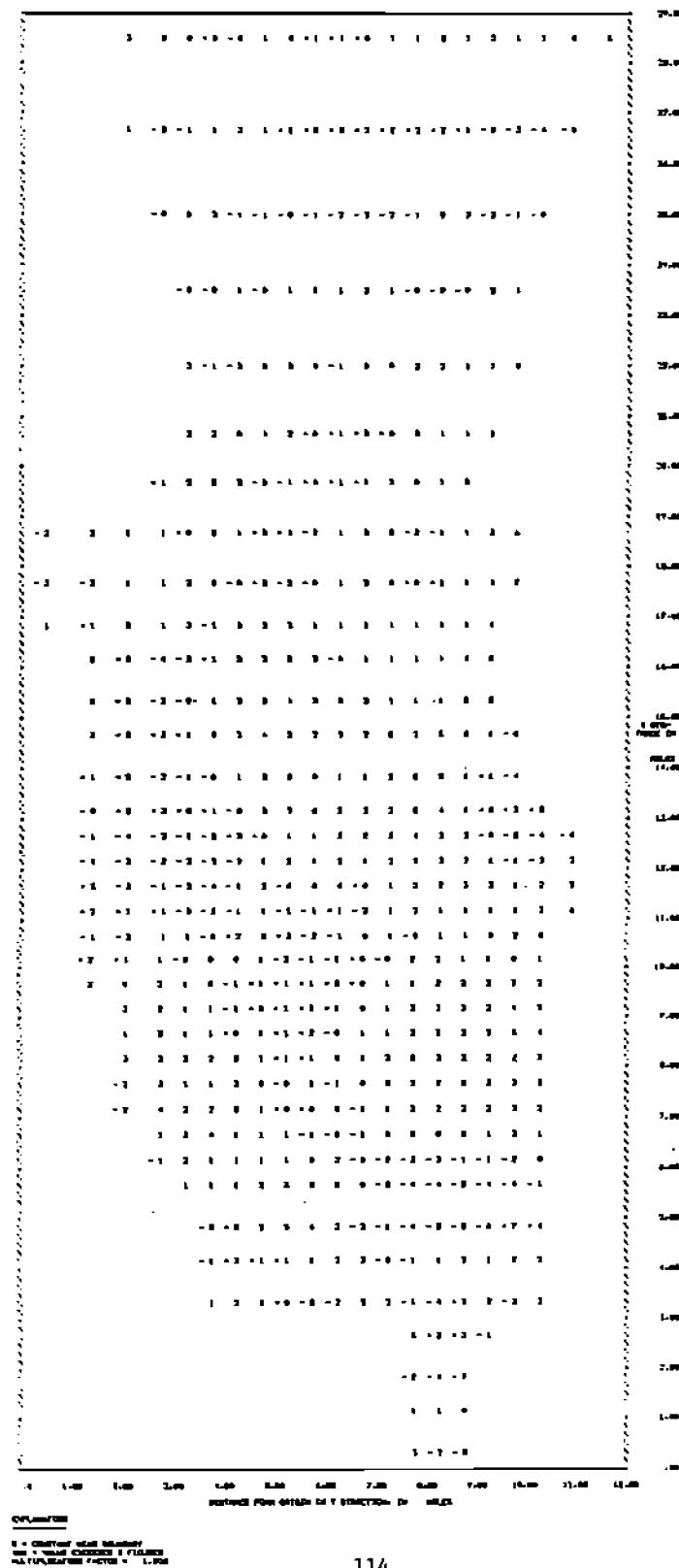
DELT IN HOURS = 8760.000

MULTIPLIER FOR DELT = 1.000
 485 WELLS

K	I	J	PUMPING RATE
1	12	25	-1.95070
1	13	25	-1.70139
1	14	25	-1.71683
1	15	19	-1.78424
1	14	20	-1.22342

Appendix E
Steady-state Drawdown Difference Map

PLOT OF DRAWDOWN DIFFERENCE LAYER 1



PLOT OF DRAWDOWN DIFFERENCE LAYER 2

